



SIGNAL WARRANT ANALYSIS

FOR

KANNER CPUD

Kanner Highway and Proposed Signalized Driveway

Prepared for:

Mr. Joe Marino
M & M Retail Partners, LLC.
Lodi, NJ 07644
973-779-7466

Prepared by:

O'Rourke Engineering & Planning
22 SE Seminole Street
Stuart, Florida 34994
(772) 781-7918

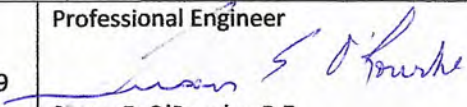
June 26, 2029

Revised February 18, 2021

Revised March 16, 2021

Revised April 22, 2021

MR18031.0

Prepared by: O'Rourke Engineering & Planning Certificate of Authorization: #26869 22 SE Seminole Street Stuart, Florida 34994 772-781-7918	Professional Engineer  Susan E. O'Rourke, P.E. Date signed and sealed: 4/22/2021 License #: 42684
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Introduction

O'Rourke Engineering & Planning prepared the signal warrant analysis of the proposed full access driveway (Driveway 1) to be referred to as Costco Boulevard, located approximately 2,400 feet north of Indian Street on Kanner Highway. **Figure 1** shows the general project location and the relative location of the proposed full access driveway which is the subject of this analysis.

Attachment A includes the project site plan.

This signal warrant analysis uses existing hourly count data on Kanner Highway and estimated hourly volumes on the minor approach, Costco Boulevard. Each component of the analysis is discussed below.

Trip Generation

Three major developments will have access to Costco Boulevard; the subject property, M&M Retail, Certus ALF and out parcel, and Cleveland Clinic Medical Office. M&M Retail will consist of 157,531 square feet of Discount Warehouse, 16,750 square feet of Shopping Center, 2,500 square feet of fast-food w/ drive-thru, 16,240 square feet of restaurant and 398 apartment units.

The minor street traffic was estimated from the future developments: M&M Retail and Cleveland Clinic Medical Office. The traffic from the Certus ALF and outparcel were not included in the analysis, as the construction of this development could occur after the development of M&M Retail. The volumes were then used to evaluate the signal warrants. Each step is discussed below.

Table 1 includes the daily trip generation for each development referenced above.

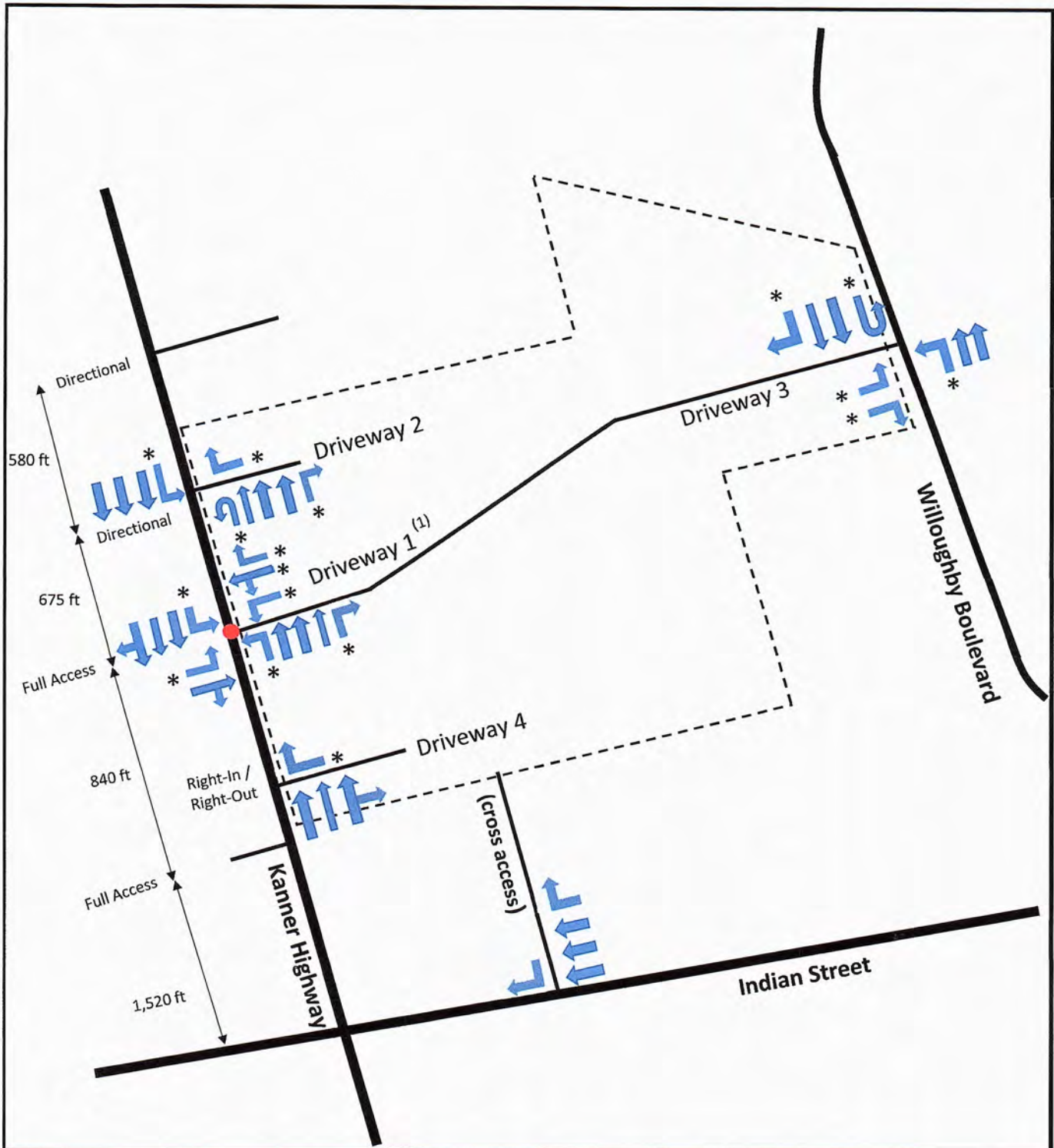
Major Street Traffic

Hourly directional traffic volumes were obtained from the FDOT website for the "Major Street": SR-76 (Kanner Highway). **Table 2** summarizes the existing hourly traffic volumes (2019) on Kanner Highway directly from the FDOT Peak Season 2019 count. The Kanner Highway volumes were compared to the Major Street thresholds. The existing major street volumes far exceed the maximum volume for several hours of the day. Given that the major street traffic is already so high before growth and before other project traffic was added, the existing traffic was used for the major street traffic.

Minor Street Traffic

The estimated movements from Costco Boulevard (minor approach) were developed based on the project traffic assigned to Costco Boulevard. **Attachment B** includes the percent assignment for each development included in the signal warrant.

The development traffic assigned to Costco Boulevard was distributed over the hours in the day based on ITE hourly distribution percentages. The percentages reflect the project traffic during that hour as it relates to daily traffic. The daily trips calculated above were applied to the hourly distribution. **Table 3** summarizes the hourly minor approach volumes. These distribution values are also included in Attachment B.



  OROURKE ENGINEERING & PLANNING NTS 22 SE Seminole Street Stuart, FL, 34994 Job #: MR18031.0 Date: 4/21/21	Legend  = Existing Lane  * = Proposed Lane  = Driveway Spacing  = Proposed Signal (1) Lane Call Out Subject to FDOT Permitting	Figure 1 Lane Geometrics M & M Retail
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Table 1: Daily Trip Generation

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips			Net External Trips		
					In	Out	In	Out	In	Out	Total	In	Out	Total
Medical Office (Cleveland Clinic)	720	155,080	SR	$T = 38.45(X) - 57.62$	50%	50%	2,535	2,535	0	0	0	2,936	2,935	5,871
M&M Retail - Residential Land Use	221	398	DU	$T = 5.45(X) - 1.75$	50%	50%	1,684	1,683	76	228	304	1,008	855	1,863
M&M Retail - Total Retail Land Use		193,021	SR		50%	50%	5,685	5,683	582	830	1,812	4,703	4,853	9,556
- Shopping Center	820	16,750	SR	$LxTx(D) = 0.68(LxTx) + 3.57$	50%	50%	892	892	47	121	168	845	771	1,616
- Discount Club	850	157,531	SR	$T = 41.89(X)$	50%	50%	3,293	3,292	174	445	619	3,119	2,847	5,966
-Fast Food (w/ Drive Thru)	857	2,500	SR	$T = 470.94(X)$	50%	50%	589	588	299	104	403	290	484	774
- High-Turnover (Sit-Down) Restaurant	934	16,240	SR	$T = 112.18(X)$	50%	50%	911	911	462	160	622	449	751	1,200
TOTALS							9,706	9,701	1,058	1,058	2,116	8,647	8,643	17,290

Source: ITE 10th Edition Trip Generation Rates

COUNTY: 89
STATION: 0114
DESCRIPTION: SR 76/COLORADO AVE - SW OF MONTEREY RD (COUNTY LINK)
START DATE: 03/20/2019
START TIME: 0000

TIME	DIRECTION: N				TOTAL	DIRECTION: S				TOTAL	COMBINED TOTAL	
	1ST	2ND	3RD	4TH		1ST	2ND	3RD	4TH			
0000	36	36	28	16	116	19	12	20	19	70	186	
0100	28	20	19	20	87	7	11	7	11	36	123	
0200	14	11	4	7	36	19	16	4	11	50	86	
0300	6	10	17	16	49	14	19	19	31	83	132	
0400	17	20	25	25	87	30	37	43	50	160	247	
0500	23	38	37	74	172	74	98	85	139	396	568	
0600	72	95	148	134	449	150	172	175	206	703	1152	
0700	163	230	254	298	945	277	296	253	269	1095	2040	
0800	284	329	256	218	1087	265	186	240	198	889	1976	
0900	230	218	224	229	901	172	188	184	212	756	1657	
1000	245	224	214	224	907	215	197	224	191	827	1734	
1100	241	234	229	284	988	210	203	229	218	860	1848	
1200	245	270	259	275	1049	212	203	194	188	797	1846	
1300	209	202	238	217	866	203	234	198	179	814	1680	
1400	256	287	228	241	1012	187	248	218	222	855	1867	
1500	270	250	254	276	1050	208	239	206	222	875	1925	
1600	317	281	287	240	1125	226	203	212	244	885	2010	
1700	275	260	271	292	1098	251	215	222	215	903	2001	
1800	260	234	208	184	886	168	157	160	145	630	1516	
1900	198	175	160	124	657	176	142	120	125	563	1220	
2000	156	134	110	88	488	106	115	131	94	446	934	
2100	79	85	82	70	316	108	101	85	77	371	687	
2200	72	68	54	54	248	62	52	41	46	201	449	
2300	22	52	59	52	185	32	37	23	20	112	297	
24-HOUR TOTALS:					14804						13377	28181

DIRECTION: N		PEAK VOLUME INFORMATION		COMBINED DIRECTIONS	
HOUR	VOLUME	HOUR	DIRECTION: S VOLUME	HOUR	DIRECTIONS VOLUME
A.M.	745	700	1095	715	2149
P.M.	1545	1645	932	1545	2024
DAILY	745	700	1095	715	2149

Table 3: Minor Approach Volumes

Table 3: Minor Approach Volumes															
Hourly	Hourly Distribution			External Volumes			Percent Assignment to Costco Blvd						Minor Approach Volume		
	M&M		Cleveland Clinic	M&M		Cleveland Clinic	M&M Lefts			M&M Rights				Cleveland Clinic Lefts	Cleveland Clinic Rights
	Retail	Residential	Medical Office	Retail	Residential	Medical Office	Retail	Residential	Retail	Residential	Retail	Residential		15.0%	0%
6-7 AM	0.2%	4.0%	1.4%	10	34	41	4	7	2	5	6	0	21		
7-8 AM	1.1%	7.5%	5.4%	53	64	158	19	13	8	10	24	0	65		
8-9 AM	2.0%	6.2%	8.9%	97	53	261	34	11	15	8	39	0	96		
9-10 AM	3.6%	4.3%	10.4%	175	37	305	61	7	26	6	46	0	130		
10-11 AM	5.6%	3.7%	10.1%	272	32	286	95	6	41	5	44	0	168		
11-12 PM	8.3%	4.5%	9.1%	403	38	267	141	8	60	6	40	0	222		
12-1 PM	10.0%	4.7%	7.9%	485	40	232	170	8	73	6	35	0	253		
1-2 PM	9.3%	4.4%	8.8%	451	38	258	158	8	66	6	39	0	242		
2-3 PM	9.0%	5.4%	9.1%	437	46	267	153	9	68	7	40	0	239		
3-4 PM	8.8%	5.8%	9.2%	427	50	270	149	10	64	8	41	0	236		
4-5 PM	9.2%	8.3%	7.8%	446	71	229	156	14	67	11	34	0	243		
5-6 PM	9.3%	10.1%	5.6%	451	86	164	158	17	68	13	25	0	241		
6-7 PM	8.0%	7.9%	2.2%	388	68	65	136	14	58	10	10	0	194		
7-8 PM	6.1%	6.3%	1.3%	286	54	38	104	11	44	8	6	0	147		
8-9 PM	4.4%	5.1%	0.7%	214	44	21	75	9	32	7	3	0	107		
9-10 PM	2.9%	3.9%	0.3%	141	33	9	49	7	21	5	1	0	70		

Signal Warrants

Using the resultant volumes shown in Tables 2 and 3, the warrants with the exception of the pedestrian, accident, and school warrants were analyzed. The minor approach of the full access driveway is the westbound approach. The left-turn volumes and 50% of the right-turn volumes will be considered for triggering the need for the signal. There will be two left-turn lanes and one right-turn lane. The speed limit on Kanner Highway is 45 MPH. The thresholds will be based on two or more lanes for the major street and two or more lanes for the minor street.

Attachment C provides the Traffic signal warrant summary of the Costco Boulevard/Kanner Highway intersection at buildout of the development.

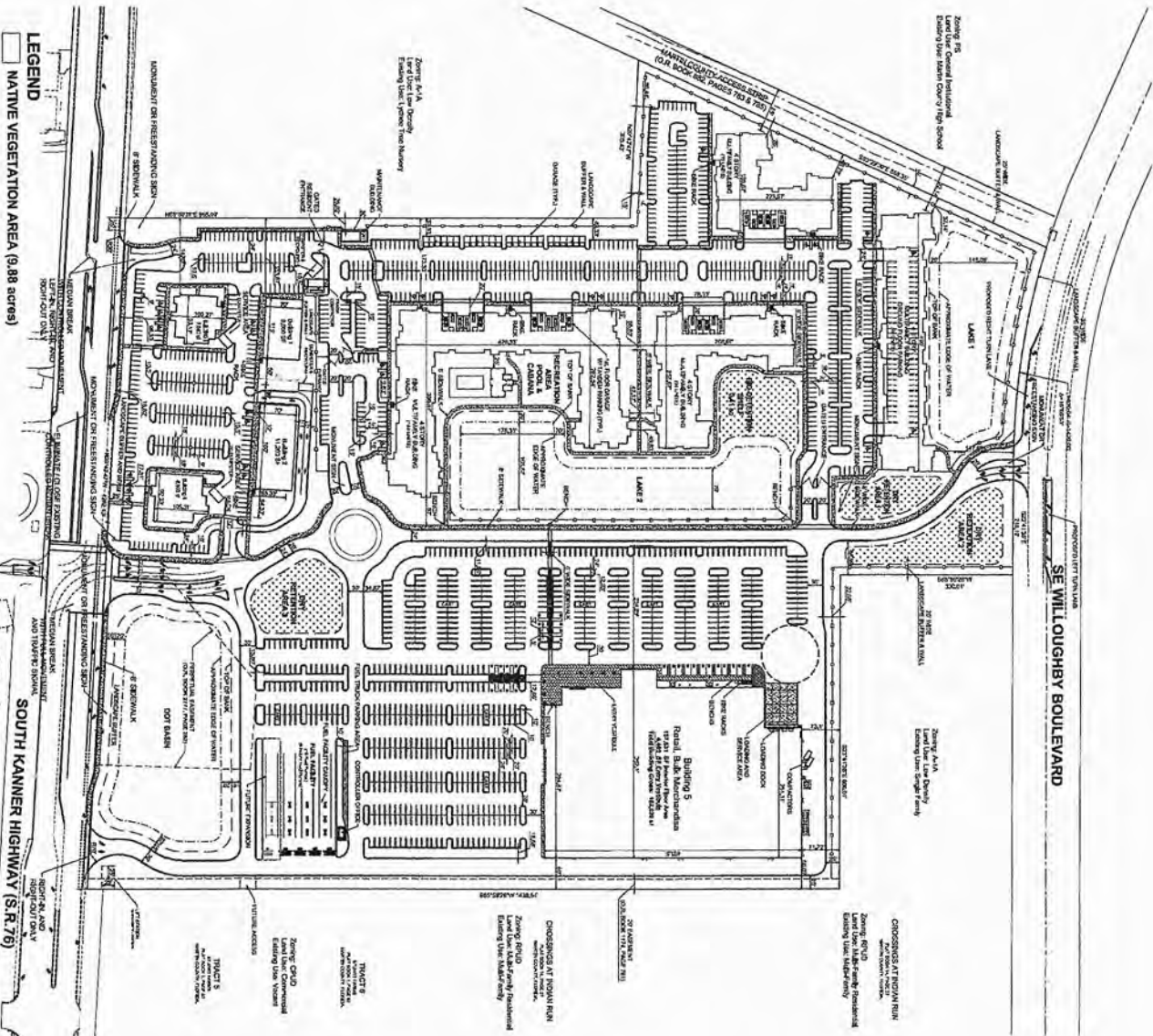
As shown, all warrants will be satisfied at 100% thresholds by completion of M&M Retail.

Conclusion

The M&M Retail development will be commencing construction once approvals are in hand first quarter 2021. The Kanner Highway and full access driveway intersection will be constructed as part of the Discount Club site plan construction. Due to the high volumes already on Kanner, the high speed of travel and the concentration of the development in the southbound left turn lane, a signal will meet monitored warrants within days of the intersection being operational. Therefore, we request that the signal be permitted for installation during the initial construction of the Kanner Highway and full access driveway intersection improvements.

ATTACHMENT A

Site Plan



Site Data

Total Site Area	2,133,087 sq ft (48.50 ac)	100%
Native Vegetation Area	9.88 ac	0.46%
Buildings	1,481 sq ft	0.07%
Parking	1,481 sq ft	0.07%
Landscaping	1,481 sq ft	0.07%
Other	1,481 sq ft	0.07%

Previous / Impervious Calculations

Impervious Area	1,525,705 sq ft (34.83 ac)	71.5%
Roofs	1,525,705 sq ft (34.83 ac)	71.5%
Driveways	1,525,705 sq ft (34.83 ac)	71.5%
Pavement	1,525,705 sq ft (34.83 ac)	71.5%
Other	1,525,705 sq ft (34.83 ac)	71.5%

Parking Requirements

Parking Required per Kanner CPUD	130,000 sq ft (3.00 ac)	6.1%
Existing Parking	130,000 sq ft (3.00 ac)	6.1%
Additional Parking	130,000 sq ft (3.00 ac)	6.1%
Other	130,000 sq ft (3.00 ac)	6.1%

Building Data

Building Coverage	370,000 sq ft (8.44 ac)	17.3%
Roofs	370,000 sq ft (8.44 ac)	17.3%
Driveways	370,000 sq ft (8.44 ac)	17.3%
Pavement	370,000 sq ft (8.44 ac)	17.3%
Other	370,000 sq ft (8.44 ac)	17.3%

Native Vegetation Requirement

Native Vegetation Area	9.88 ac	0.46%
Roofs	9.88 ac	0.46%
Driveways	9.88 ac	0.46%
Pavement	9.88 ac	0.46%
Other	9.88 ac	0.46%

Land Use Site Data

Total Site Area	2,133,087 sq ft (48.50 ac)	100%
Native Vegetation Area	9.88 ac	0.46%
Buildings	1,481 sq ft	0.07%
Parking	1,481 sq ft	0.07%
Landscaping	1,481 sq ft	0.07%
Other	1,481 sq ft	0.07%

Density & Floor Area Ratio

Density	1,481 sq ft / 0.07 ac	21,157 sq ft / ac
Floor Area Ratio	1,481 sq ft / 0.07 ac	21,157 sq ft / ac
Other	1,481 sq ft / 0.07 ac	21,157 sq ft / ac

General Notes

- All building setbacks and other requirements shall be in accordance with the City of Kanner Comprehensive Zoning Ordinance.
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luisdo & associates
 1000 S. Kanner Highway
 Kanner, FL 32053
 321.555.1234
 www.luisdo.com



Project TOWN:

Project Name	Kanner CPUD
Project Address	1000 S. Kanner Highway
Project City	Kanner, FL 32053
Project State	FL
Project Zip	32053
Project Phone	321.555.1234
Project Email	info@luisdo.com
Project Website	www.luisdo.com

KANNER CPUD City of Stuart, Martin County, Florida Master Site Plan

Date	By	Description
12/1/2020	J. Smith	Initial Design
12/1/2020	J. Smith	Final Design
12/1/2020	J. Smith	Final Design
12/1/2020	J. Smith	Final Design

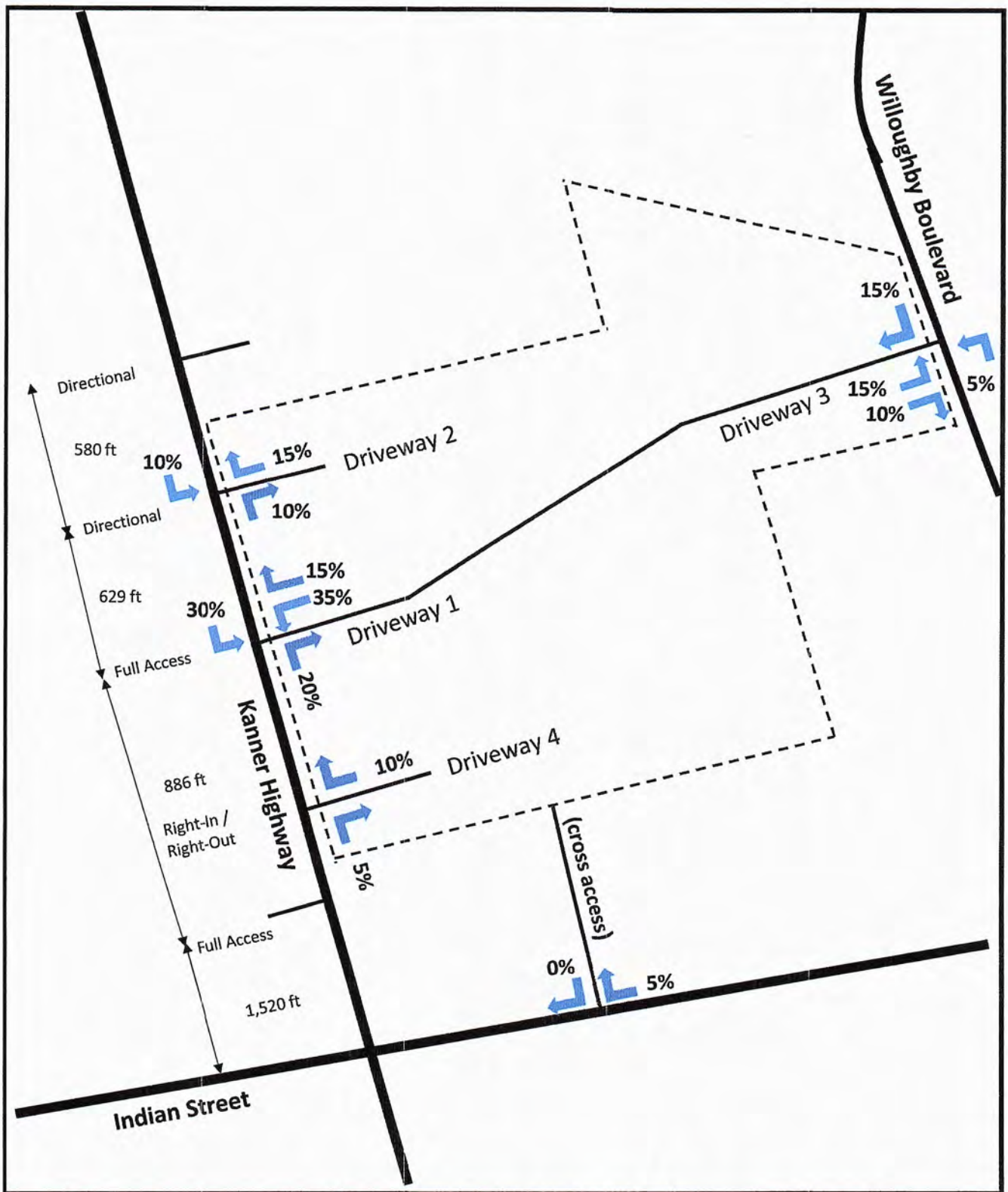


Scale: 1" = 100'

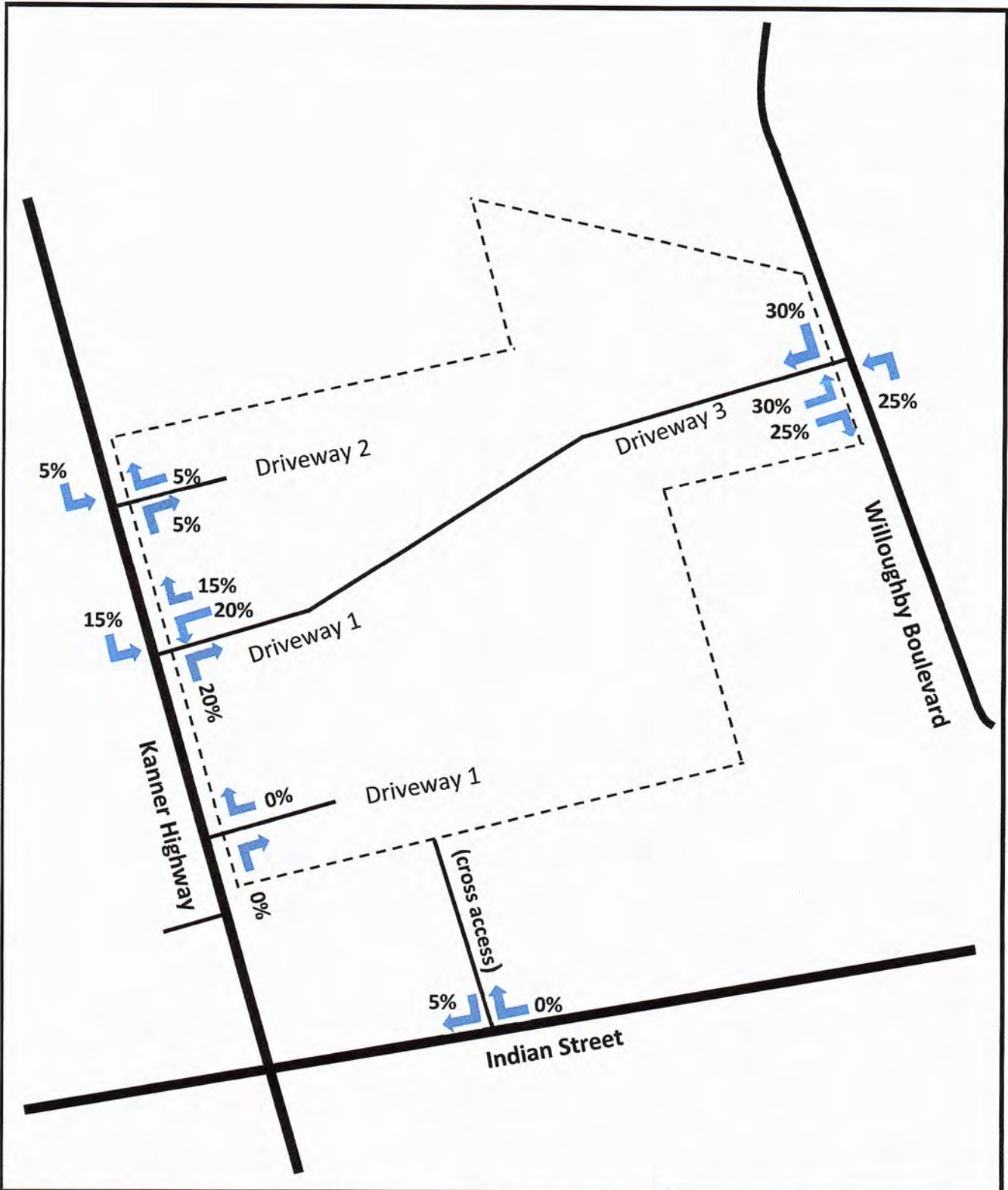
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ATTACHMENT B

**Percent Assignment
and
Distribution Percentage**



  NTS 22 SE Seminole St Stuart, FL, 34994 Job #: MR18031.0 Date: 4.22.21	Legend  = Project Location  = Direction 15% = Percent Project Traffic xx ft. = Driveway Spacing	Figure 2a Driveway Assignment - Retail M & M Retail
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  NTS 22 SE Seminole St Stuart, FL, 34994 Job #: MR18031.0 Date: 8/31/18	Legend  = Project Location  = Direction 15% = Percent Project Traffic xx ft. = Driveway Spacing	Figure 2b Driveway Assignment - Residential M & M Retail
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FIGURE - 2
Project Percent Assignment
MMHS - SB 10

LEGEND
% = Project Percent Assignment

O'ROURKE ENGINEERING & PLANNING

Land Use	221 Multifamily Housing (Mid-Rise)						222 Multifamily Housing (High-Rise)											
	General Urban/ Suburban		Dense Multi- Use Urban		Center City Core		Dense Multi-Use Urban						Center City Core					
Time Period	Weekday		Weekday		Weekday		Weekday		Saturday		Sunday		Weekday		Saturday		Sunday	
Trip Type	Vehicle		Vehicle		Vehicle		Person		Person		Person		Person		Person		Person	
# Data Sites	8		4		3		8		8		8		6		6		5	
	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
12:00	0.5	4.7	0.5	5.0	1.3	3.5	1.6	4.2	3.1	6.2	2.4	6.4	0.8	6.1	1.9	7.6	2.4	6.8
12:15	0.4	4.6	0.7	5.0	1.3	4.4	1.3	4.4	2.8	6.0	2.1	6.1	0.8	5.8	1.8	7.5	2.0	6.6
12:30	0.3	4.3	0.5	5.3	0.7	4.6	1.1	4.3	2.5	5.9	1.9	6.0	0.7	5.9	1.3	7.2	1.8	6.6
12:45	0.3	4.5	0.7	4.8	0.4	4.4	0.8	4.5	2.2	5.9	1.6	5.9	0.4	5.4	1.1	7.4	1.6	6.8
1:00	0.3	4.4	0.7	4.5	0.2	5.3	0.7	4.7	1.8	5.8	1.4	6.1	0.3	5.3	1.0	6.6	1.5	6.6
1:15	0.3	4.5	0.5	4.4	0.2	4.6	0.5	4.5	1.4	6.1	1.4	6.3	0.2	5.3	0.9	6.3	1.3	7.1
1:30	0.3	5.0	0.7	3.8	0.7	4.6	0.4	4.9	1.2	6.1	1.3	6.8	0.2	5.3	0.8	6.2	1.0	7.0
1:45	0.3	5.2	0.5	4.7	0.7	4.2	0.4	5.2	1.0	6.3	1.2	7.2	0.2	5.3	0.7	6.2	0.9	6.9
2:00	0.2	5.4	0.6	4.6	0.9	4.4	0.3	5.4	0.9	6.3	1.2	7.0	0.2	4.7	0.9	6.2	0.8	6.8
2:15	0.2	5.4	0.7	4.4	0.9	4.9	0.4	5.7	0.9	6.0	0.9	6.9	0.1	4.7	0.9	6.1	0.6	6.7
2:30	0.2	5.3	0.7	5.2	0.7	4.6	0.4	5.9	0.7	6.3	0.7	6.4	0.2	5.2	0.8	6.1	0.7	6.7
2:45	0.3	5.2	0.6	5.2	0.4	5.3	0.4	5.8	0.8	6.5	0.8	6.3	0.3	5.5	0.8	6.1	0.5	6.7
3:00	0.3	5.8	0.5	5.3	0.2	5.1	0.3	5.6	0.7	6.6	0.6	6.4	0.3	6.1	0.6	6.5	0.4	6.6
3:15	0.4	6.5	0.2	5.8	0.2	4.6	0.3	6.1	0.7	6.5	0.6	6.4	0.3	6.3	0.4	6.8	0.5	6.4
3:30	0.4	7.1	0.2	6.0	0.2	5.7	0.3	5.9	0.7	6.4	0.6	6.6	0.4	6.1	0.4	6.8	0.4	6.4
3:45	0.5	7.9	0.3	7.2	1.1	4.9	0.4	6.0	0.7	6.4	0.5	6.4	0.3	6.0	0.4	6.8	0.4	6.0
4:00	0.5	8.3	0.3	7.4	1.1	5.3	0.6	6.5	0.7	6.5	0.6	7.0	0.4	5.8	0.5	7.0	0.4	6.0
4:15	0.8	9.4	0.5	8.2	1.3	6.8	0.7	6.1	0.6	7.0	0.5	7.4	0.5	5.7	0.4	6.7	0.3	6.1
4:30	1.1	9.8	1.1	8.0	1.5	6.6	0.9	6.6	0.6	7.4	0.5	7.8	0.6	6.0	0.4	7.0	0.3	6.5
4:45	1.5	10.1	1.4	7.0	1.3	7.3	1.0	7.0	0.5	7.3	0.5	8.1	0.8	6.4	0.4	6.6	0.2	7.0
5:00	1.7	10.1	1.9	6.5	1.8	7.9	1.2	7.0	0.6	7.6	0.6	7.7	0.9	6.6	0.4	6.9	0.3	7.3
5:15	1.8	9.5	2.2	6.3	2.0	6.0	1.4	7.5	0.6	7.3	0.7	7.6	1.0	7.2	0.5	7.1	0.3	7.3
5:30	2.3	9.0	2.8	6.5	2.2	6.2	1.7	8.1	0.7	7.1	0.8	7.3	1.2	7.2	0.6	6.5	0.4	6.5
5:45	3.0	8.5	3.4	6.3	2.6	7.5	2.0	7.9	0.8	7.1	0.9	6.8	1.5	7.5	0.6	6.5	0.6	7.2
6:00	4.0	7.9	4.1	7.7	4.9	6.4	2.6	7.9	1.0	6.8	1.0	7.0	2.0	7.9	0.7	6.1	0.7	7.3
6:15	5.1	7.2	5.2	7.1	6.8	7.1	3.5	7.7	1.2	6.6	1.0	6.5	2.7	7.8	1.1	6.1	1.0	7.1
6:30	6.0	6.7	6.0	7.0	7.7	7.1	4.2	7.2	1.5	6.5	1.0	7.0	3.4	7.7	1.3	6.6	1.2	7.6
6:45	6.8	6.3	9.2	6.5	9.3	5.5	5.5	6.9	1.8	6.3	1.2	7.3	4.2	7.8	1.6	6.8	1.4	6.9
7:00	7.5	6.3	10.9	5.5	8.6	6.2	6.8	6.7	2.1	6.0	1.3	7.0	4.7	7.6	2.1	6.6	1.7	6.6
7:15	7.5	6.1	12.0	5.3	7.5	6.4	7.8	6.5	2.5	5.8	1.6	6.8	5.8	7.1	2.1	6.6	2.1	6.6
7:30	7.5	6.1	11.8	5.0	7.9	5.7	8.8	6.2	2.7	5.6	2.0	6.2	7.1	6.7	2.7	6.1	2.4	6.4
7:45	6.9	5.9	9.0	5.1	7.7	5.3	8.5	6.0	3.1	5.5	2.4	5.7	7.8	5.9	3.1	5.6	2.8	6.2
8:00	6.2	5.1	7.2	4.7	7.5	4.6	8.0	5.7	3.6	5.5	2.8	5.6	8.1	5.9	3.3	5.4	3.2	6.0
8:15	5.7	4.7	5.8	4.2	7.1	4.9	6.8	5.5	4.1	5.5	3.3	5.5	7.8	5.5	3.9	4.7	3.7	5.6
8:30	4.9	4.4	4.7	4.3	6.4	4.9	6.1	5.1	4.5	5.3	3.7	5.0	7.2	5.3	4.1	4.4	4.1	5.1
8:45	4.6	4.1	4.8	4.0	5.7	5.7	5.8	4.7	4.9	5.1	3.7	4.8	7.0	5.3	5.1	4.4	4.7	5.0
9:00	4.3	3.9	4.0	3.8	5.3	5.1	5.1	4.4	5.0	4.9	4.1	4.5	6.7	4.7	5.6	4.1	5.0	4.7
9:15	4.1	3.6	3.9	4.1	4.9	4.4	5.1	3.8	4.7	4.8	4.5	4.8	6.1	4.5	5.9	4.4	5.2	4.4
9:30	4.0	3.3	3.7	3.7	5.1	5.3	4.7	3.5	4.6	4.5	4.8	4.9	5.4	4.4	6.3	4.3	5.3	4.1
9:45	3.8	2.8	3.6	3.7	4.2	4.9	4.3	3.4	4.6	4.3	5.3	5.3	4.9	3.8	6.1	4.1	5.7	3.5
10:00	3.7	2.7	4.1	3.6	3.5	5.1	4.3	3.1	4.8	4.2	5.6	5.1	4.6	3.5	6.4	4.1	6.2	3.3
10:15	3.8	2.5	4.3	3.4	3.3	4.6	4.4	3.4	5.2	4.1	5.6	4.6	4.2	3.3	6.4	3.4	6.8	3.1
10:30	4.0	2.2	4.2	3.0	2.4	3.3	4.3	3.1	5.8	4.0	5.5	4.1	4.4	2.7	6.3	3.4	7.5	2.5
10:45	4.3	1.9	3.8	2.5	2.9	2.4	4.2	2.9	5.7	3.9	5.6	3.4	4.6	2.4	6.6	3.3	7.5	2.3
11:00	4.5	1.5	4.4	2.3	3.3	2.4	4.5	2.8	5.6	3.6	5.8	3.0	4.9	2.1	6.6	2.9	7.5	2.0
11:15	4.7	1.2	4.3	1.6	4.0	1.8	4.4	2.3	6.0	3.5	6.2	2.8	5.5	1.6	7.2	2.6	7.1	2.2
11:30	4.9	0.9	4.6	1.2	4.0	1.8	4.4	2.1	5.8	3.4	6.3	2.5	5.4	1.2	7.7	2.5	6.9	2.5
11:45	4.6	0.6	4.8	0.9	4.4	1.8	4.3	1.9	6.0	3.2	6.5	2.5	5.8	1.1	7.3	2.2	6.9	2.5

Land Use Setting Time Period Trip Type # Data Sites	820 Shopping Center		840 Automobile Sales (New)		841 Automobile Sales (Used)		842 Recreational Vehicle Sales		843 Automobile Parts Sales		848 Tire Store	
	General Urban/ Suburban		General Urban/ Suburban		General Urban/ Suburban		General Urban/ Suburban		General Urban/ Suburban		General Urban/ Suburban	
	Weekday		Weekday		Weekday		Weekday		Weekday		Weekday	
	Vehicle		Vehicle		Vehicle		Vehicle		Vehicle		Vehicle	
	10		6		14		5		7		6	
	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
12:00	0.2	10.0	0.0	9.4	0.0	9.1	0.0	10.9	0.0	8.6	0.0	7.4
12:15	0.1	10.1	0.0	8.7	0.0	9.9	0.0	10.5	0.0	8.9	0.0	8.7
12:30	0.2	9.8	0.0	9.0	0.0	9.9	0.0	10.7	0.0	9.2	0.0	9.4
12:45	0.1	9.6	0.0	8.9	0.0	9.4	0.0	8.7	0.0	9.8	0.0	11.3
1:00	0.1	9.3	0.0	9.5	0.0	8.3	0.0	8.3	0.0	8.6	0.0	11.8
1:15	0.1	9.2	0.0	9.8	0.0	7.5	0.0	8.5	0.0	7.8	0.0	11.9
1:30	0.0	9.2	0.0	10.2	0.0	7.1	0.0	7.7	0.0	7.4	0.0	12.9
1:45	0.0	9.2	0.0	10.2	0.0	6.3	0.0	8.2	0.0	6.7	0.0	11.0
2:00	0.0	9.0	0.0	10.0	0.0	7.7	0.0	8.2	0.0	7.0	0.0	10.4
2:15	0.0	8.8	0.0	9.8	0.0	8.5	0.0	9.0	0.0	7.8	0.0	9.1
2:30	0.0	8.8	0.0	9.1	0.0	8.5	0.0	8.0	0.0	8.4	0.0	8.3
2:45	0.0	8.8	0.0	8.9	0.0	8.6	0.0	7.8	0.0	9.0	0.0	9.3
3:00	0.0	8.8	0.0	8.3	0.0	8.4	0.0	7.8	0.0	8.7	0.0	10.1
3:15	0.0	8.9	0.0	8.1	0.0	8.9	0.0	7.0	0.0	8.7	0.0	10.0
3:30	0.0	9.0	0.0	8.1	0.0	9.8	0.0	9.2	0.0	8.4	0.0	9.4
3:45	0.0	9.0	0.0	7.7	0.0	10.9	0.0	9.0	0.0	8.2	0.0	9.5
4:00	0.0	9.2	0.0	7.4	0.0	11.0	0.0	10.0	0.0	8.8	0.0	9.4
4:15	0.0	9.2	0.0	7.5	0.0	11.1	0.0	9.3	0.0	8.5	0.0	9.1
4:30	0.1	9.3	0.0	8.0	0.0	11.4	0.0	8.0	0.0	8.2	0.2	8.4
4:45	0.1	9.4	0.0	8.1	0.0	10.8	0.0	8.8	0.0	7.6	0.2	8.3
5:00	0.1	9.3	0.0	8.2	0.0	10.2	0.0	6.7	0.0	7.5	0.4	6.9
5:15	0.1	9.1	0.0	8.1	0.0	9.6	0.0	5.8	0.0	7.3	0.5	6.1
5:30	0.2	8.7	0.1	6.8	0.0	7.7	0.0	4.3	0.0	7.0	0.5	5.4
5:45	0.2	8.3	0.2	5.9	0.0	6.3	0.0	2.0	0.1	6.7	0.5	3.8
6:00	0.2	8.0	0.7	5.0	0.0	4.5	0.0	1.3	0.3	6.6	0.4	3.0
6:15	0.4	7.6	1.7	4.1	0.0	3.2	0.3	0.8	0.8	5.5	0.7	2.3
6:30	0.6	7.1	2.9	3.6	0.2	2.4	0.3	0.7	1.0	5.3	1.0	1.9
6:45	0.8	6.7	4.3	3.0	0.7	1.8	2.3	0.5	1.5	5.5	1.8	1.4
7:00	1.1	6.1	5.7	2.9	1.9	1.9	5.5	0.2	2.1	5.0	2.9	1.2
7:15	1.4	5.8	6.6	1.7	3.7	1.7	6.5	0.0	2.5	5.4	3.4	0.7
7:30	1.6	5.5	7.8	1.2	4.3	1.6	8.5	0.0	4.3	5.5	4.9	0.3
7:45	1.8	4.9	8.2	0.7	4.9	1.6	7.0	0.0	4.8	5.2	6.6	0.5
8:00	2.0	4.4	8.3	0.0	5.0	1.1	6.3	0.0	5.5	4.3	7.2	0.5
8:15	2.3	4.2	8.4	0.0	7.2	0.4	9.3	0.0	6.6	3.5	7.7	0.3
8:30	2.6	3.8	7.9	0.0	7.7	0.2	9.5	0.0	6.7	2.7	7.8	0.3
8:45	3.1	3.4	8.2	0.0	9.0	0.0	12.5	0.0	7.6	1.9	8.3	0.0
9:00	3.6	2.9	7.5	0.0	11.0	0.0	14.0	0.0	8.2	1.7	9.4	0.0
9:15	4.1	2.3	7.8	0.0	9.7	0.0	12.2	0.0	8.1	1.3	10.9	0.0
9:30	4.7	1.7	7.6	0.0	10.4	0.0	12.7	0.0	7.8	0.8	11.3	0.0
9:45	5.1	1.4	7.8	0.0	11.0	0.0	12.4	0.0	8.0	0.5	10.9	0.0
10:00	5.6	1.1	8.2	0.0	10.2	0.0	12.4	0.0	8.4	0.1	10.7	0.0
10:15	6.3	0.8	8.4	0.0	9.7	0.0	12.5	0.0	8.0	0.0	9.9	0.0
10:30	6.9	0.7	8.7	0.0	10.4	0.0	11.2	0.0	8.5	0.0	9.2	0.0
10:45	7.6	0.5	8.5	0.0	9.7	0.0	11.7	0.0	8.5	0.0	8.9	0.0
11:00	8.3	0.5	8.8	0.0	9.8	0.0	8.3	0.0	8.6	0.0	8.3	0.0
11:15	8.9	0.4	9.4	0.0	9.0	0.0	8.0	0.0	9.3	0.0	8.5	0.0
11:30	9.5	0.3	9.1	0.0	8.5	0.0	9.2	0.0	8.8	0.0	8.6	0.0
11:45	9.8	0.3	9.4	0.0	9.0	0.0	9.0	0.0	8.4	0.0	7.7	0.0

Land Use Setting Time Period Trip Type # Data Sites	720 Medical-Dental Office Building						730 Government Office Building		731 State Motor Vehicles Department	
	General Urban/Suburban						General Urban/Suburban		General Urban/Suburban	
	Weekday Vehicle		Saturday Vehicle		Sunday Vehicle		Weekday Vehicle		Weekday Vehicle	
	19		1		1		7		1	
	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
12:00	0.1	7.9	0.0	17.9	0.0	20.0	0.0	9.8	0.0	8.9
12:15	0.1	7.9	0.0	21.6	0.0	20.0	0.0	9.3	0.0	12.1
12:30	0.1	8.0	0.0	18.7	0.0	10.0	0.0	9.5	0.0	13.1
12:45	0.1	8.4	0.0	14.9	0.0	0.0	0.0	9.3	0.0	11.9
1:00	0.1	8.8	0.0	10.4	0.0	0.0	0.0	8.8	0.0	10.8
1:15	0.1	8.9	0.0	0.0	0.0	0.0	0.0	9.2	0.4	8.5
1:30	0.0	9.3	0.0	0.0	0.0	0.0	0.0	8.8	0.4	7.5
1:45	0.1	9.2	0.0	0.0	0.0	0.0	0.0	9.5	0.4	8.3
2:00	0.1	9.1	0.0	0.0	0.0	20.0	0.0	9.8	0.4	8.7
2:15	0.1	9.3	0.0	0.0	0.0	20.0	0.0	8.5	0.0	8.3
2:30	0.1	9.2	0.0	0.0	0.0	20.0	0.0	9.3	0.0	7.9
2:45	0.0	9.3	0.0	0.0	0.0	20.0	0.0	8.8	0.0	7.9
3:00	0.1	9.2	0.0	0.0	0.0	0.0	0.0	8.2	0.0	7.9
3:15	0.2	8.7	0.0	0.0	0.0	0.0	0.0	8.8	0.4	6.0
3:30	0.4	8.6	0.0	0.0	0.0	0.0	0.0	7.3	0.4	5.0
3:45	0.5	8.1	0.0	0.0	0.0	0.0	0.0	6.3	0.8	9.4
4:00	0.6	7.8	0.0	0.0	0.0	0.0	0.0	6.3	1.2	8.7
4:15	0.6	8.3	0.0	0.0	0.0	0.0	0.0	8.9	0.8	10.2
4:30	0.5	7.4	0.0	0.0	0.0	0.0	0.0	8.2	0.8	10.0
4:45	0.4	6.7	0.0	0.0	0.0	0.0	0.0	6.9	0.4	5.6
5:00	0.4	5.6	0.0	0.0	0.0	0.0	0.0	5.5	0.0	5.2
5:15	0.5	4.2	0.0	0.0	0.0	0.0	0.0	1.1	0.0	4.6
5:30	0.7	3.5	0.0	0.0	20.0	0.0	0.0	0.4	0.0	5.8
5:45	0.9	2.7	0.0	0.0	20.0	0.0	0.0	0.3	0.0	5.8
6:00	1.4	2.2	3.7	0.0	30.0	0.0	0.2	0.0	0.8	8.9
6:15	2.0	1.7	4.5	0.0	30.0	0.0	1.3	0.0	1.2	7.5
6:30	2.8	1.5	4.5	0.0	10.0	0.0	1.8	0.0	1.2	6.2
6:45	4.0	1.4	4.5	0.0	10.0	0.0	3.3	0.0	2.7	5.6
7:00	5.4	1.3	3.0	0.0	0.0	0.0	7.1	0.0	5.2	2.5
7:15	6.7	1.1	5.2	0.0	0.0	0.0	9.3	0.0	8.1	3.3
7:30	7.6	1.1	6.7	0.0	0.0	0.0	12.0	0.0	10.0	3.7
7:45	8.2	1.0	8.2	0.0	0.0	0.0	13.8	0.0	9.8	2.9
8:00	8.9	0.7	9.0	0.0	0.0	0.0	12.4	0.0	8.7	2.5
8:15	9.3	0.6	9.7	0.0	0.0	0.0	11.8	0.0	7.3	1.7
8:30	9.7	0.5	11.2	0.0	0.0	0.0	10.7	0.0	7.3	0.4
8:45	10.3	0.4	11.9	0.0	0.0	0.0	9.5	0.0	8.9	0.0
9:00	10.4	0.3	11.2	0.0	10.0	0.0	8.5	0.0	7.5	0.0
9:15	10.7	0.2	12.7	0.0	10.0	0.0	8.5	0.0	7.1	0.0
9:30	10.7	0.2	17.9	0.0	20.0	0.0	8.1	0.0	6.7	0.0
9:45	10.5	0.2	21.6	0.0	30.0	0.0	8.8	0.0	5.0	0.0
10:00	10.1	0.2	25.4	0.0	20.0	0.0	10.1	0.0	5.4	0.0
10:15	9.9	0.1	23.9	0.0	20.0	0.0	10.2	0.0	5.2	0.0
10:30	9.7	0.1	21.6	0.0	10.0	0.0	12.5	0.0	4.8	0.0
10:45	9.5	0.2	20.1	0.0	0.0	0.0	12.9	0.0	6.2	0.0
11:00	9.1	0.1	19.4	0.0	0.0	0.0	13.4	0.0	6.4	0.0
11:15	8.8	0.2	22.4	0.0	0.0	0.0	13.2	0.0	7.3	0.0
11:30	8.3	0.1	19.4	0.0	10.0	0.0	11.4	0.0	8.7	0.0
11:45	8.0	0.0	18.7	0.0	20.0	0.0	10.6	0.0	8.3	0.0

ATTACHMENT C

Traffic Signal Warrant Summary

State of Florida Department of Transportation
TRAFFIC SIGNAL WARRANT SUMMARY

City: Stuart
County: 89 – Martin
District: Four

Engineer: James Kemp
Date: November 25, 2020

Major Street: Kanner Highway
Minor Street: M&M Full Access Dwy (Costco Blvd)

Lanes: 6 Major Approach Speed: 45
Lanes: 3 Minor Approach Speed: 35

MUTCD Electronic Reference to Chapter 4: <http://mutcd.fhwa.dot.gov/pdfs/2009r1r2/part4.pdf>

Volume Level Criteria

1. Is the posted speed or 85th-percentile of major street > 40 mph (70 km/h)? ☒ Yes ☐ No
2. Is the intersection in a built-up area of an isolated community with a population < 10,000? ☐ Yes ☒ No
"70%" volume level may be used if Question 1 or 2 above is answered "Yes" ☒ 70% ☒ 100%

WARRANT 1 - EIGHT-HOUR VEHICULAR VOLUME

Warrant 1 is satisfied if Condition A or Condition B is "100%" satisfied for eight hours. ☒ Yes ☐ No

Warrant 1 is also satisfied if both Condition A and Condition B are "80%" satisfied (should only be applied after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems). ☒ Yes ☐ No

Condition A - Minimum Vehicular Volume

Condition A is intended for application at locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal.

100% Satisfied: ☐ Yes ☒ No
80% Satisfied: ☒ Yes ☐ No
70% Satisfied: ☒ Yes ☐ No

Number of Lanes for moving traffic on each approach		Vehicles per hour on major-street (total of both approaches)			Vehicles per hour on minor-street (one direction only)		
Major	Minor	100% ^a	80% ^b	70% ^c	100% ^a	80% ^b	70% ^c
1	1	500	400	350	150	120	105
2 or more	1	600	480	420	150	120	105
2 or more	2 or more	600	480	420	200	160	140
1	2 or more	500	400	350	200	160	140

^a Basic Minimum hourly volume

^b Used for combination of Conditions A and B after adequate trial of other remedial measures

^c May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

Record 8 highest hours and the corresponding major-street and minor-street volumes in the Instructions Sheet.

Street	Eight Highest Hours							
	11:00 AM	12:00 PM	1:00 PM	2:00 PM	3:00 PM	4:00 PM	5:00 PM	6:00 PM
Major	1,848	1,846	1,680	1,867	1,925	2,010	2,001	1,516
Minor	222	253	242	239	236	243	241	194

Existing Volumes

State of Florida Department of Transportation
TRAFFIC SIGNAL WARRANT SUMMARY

Form 750-020-01
TRAFFIC ENGINEERING
10/15

Condition B - Interruption of Continuous Traffic

Condition B is intended for application where Condition A is not satisfied and the traffic volume on a major street is so heavy that traffic on the minor intersecting street suffers excessive delay or conflict in entering or crossing the major street.

Applicable: ☒ Yes ☐ No
100% Satisfied: ☒ Yes ☐ No
80% Satisfied: ☒ Yes ☐ No
70% Satisfied: ☒ Yes ☐ No

Number of Lanes for moving traffic on each approach		Vehicles per hour on major-street (total of both approaches)			Vehicles per hour on minor-street (one direction only)		
Major	Minor	100% ^a	80% ^b	70% ^c	100% ^a	80% ^b	70% ^c
1	1	750	600	525	75	60	53
2 or more	1	900	720	630	75	60	53
2 or more	2 or more	900	720	630	100	80	70
1	2 or more	750	600	525	100	80	70

^a Basic Minimum hourly volume

^b Used for combination of Conditions A and B after adequate trial of other remedial measures

^c May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

Record 8 highest hours and the corresponding major-street and minor-street volumes in the Instructions Sheet.

Eight Highest Hours								
Street	11:00 AM	12:00 PM	1:00 PM	2:00 PM	3:00 PM	4:00 PM	5:00 PM	6:00 PM
Major	1,848	1,846	1,680	1,867	1,925	2,010	2,001	1,516
Minor	222	253	242	239	236	243	241	194

Existing Volumes

State of Florida Department of Transportation
TRAFFIC SIGNAL WARRANT SUMMARY

Form 750-020-01
TRAFFIC ENGINEERING
10/15

City: Stuart
County: 89 – Martin
District: Four

Engineer: James Kemp
Date: November 25, 2020

Major Street: Kanner Highway
Minor Street: M&M Full Access Dwy (Costco Blvd)

Lanes: 6 Major Approach Speed: 45
Lanes: 3 Minor Approach Speed: 35

MUTCD Electronic Reference to Chapter 4: <http://mutcd.fhwa.dot.gov/pdfs/2009r1r2/part4.pdf>

Volume Level Criteria

1. Is the posted speed or 85th-percentile of major street > 40 mph (70 km/h)? ☒ Yes ☐ No
2. Is the intersection in a built-up area of an isolated community with a population < 10,000? ☐ Yes ☒ No
- "70%" volume level may be used if Question 1 or 2 above is answered "Yes" ☒ Yes ☐ No

WARRANT 2 - FOUR-HOUR VEHICULAR VOLUME

If all four points lie above the appropriate line, then the warrant is satisfied.

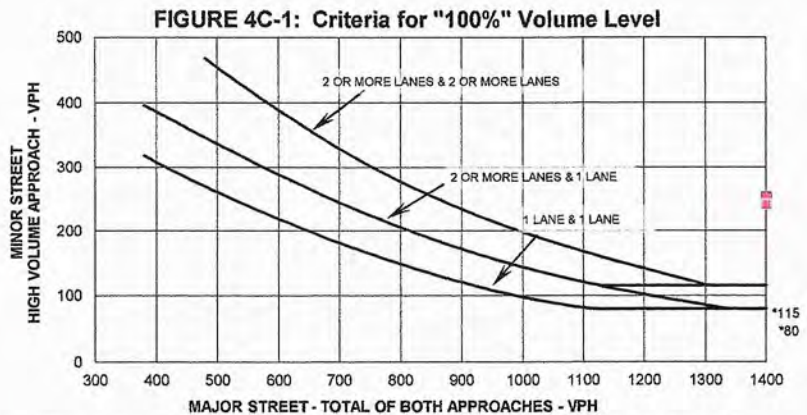
Applicable: ☒ Yes ☐ No

Satisfied: ☒ Yes ☐ No

Plot four volume combinations on the applicable figure below.

100% Volume Level

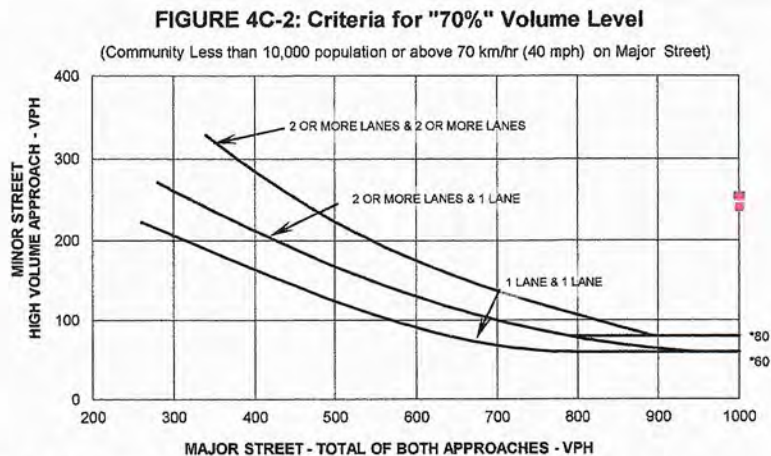
Four Highest Hours	Volumes	
	Major Street	Minor Street
12:00 PM	1846	253
1:00 PM	1680	242
4:00 PM	2010	243
5:00 PM	2001	241



* Note: 115 vph applies as the lower threshold volume for a minor street approach with two or more lanes and 80 vph applies as the lower threshold volume threshold for a minor street approach with one lane.

70% Volume Level

Four Highest Hours	Volumes	
	Major Street	Minor Street
12:00 PM	1846	253
1:00 PM	1680	242
4:00 PM	2010	243
5:00 PM	2001	241



* Note: 80 vph applies as the lower threshold volume for a minor street approach with two or more lanes and 60 vph applies as the lower threshold volume threshold for a minor street approach with one lane.

State of Florida Department of Transportation
TRAFFIC SIGNAL WARRANT SUMMARY

Form 750-020-01
TRAFFIC ENGINEERING
10/15

City: Stuart
County: 89 – Martin
District: Four

Engineer: James Kemp
Date: November 25, 2020

Major Street: Kanner Highway
Minor Street: M&M Full Access Dwy (Costco Blvd)

Lanes: 6 Major Approach Speed: 45
Lanes: 3 Minor Approach Speed: 35

MUTCD Electronic Reference to Chapter 4: <http://mutcd.fhwa.dot.gov/pdfs/2009r1r2/part4.pdf>

Volume Level Criteria

1. Is the posted speed or 85th-percentile of major street > 40 mph (70 km/h)? ☒ Yes ☐ No
 2. Is the intersection in a built-up area of an isolated community with a population < 10,000? ☐ Yes ☒ No
- "70%" volume level may be used if Question 1 or 2 above is answered "Yes" ☒ 70% ☒ 100%

WARRANT 3 - PEAK HOUR

If all three criteria are fulfilled or the plotted point lies above the appropriate line, then the warrant is satisfied.

Applicable: ☒ Yes ☐ No
Satisfied: ☒ Yes ☐ No

Unusual condition justifying use of warrant:

Record hour when criteria are fulfilled and the corresponding delay or volume in boxes provided.

Peak Hour 100% Volume		
Time	Major Vol.	Minor Vol.
12:00 PM	1846	253

Peak Hour 70% Volume		
Time	Major Vol.	Minor Vol.
12:00 PM	1846	253

Criteria

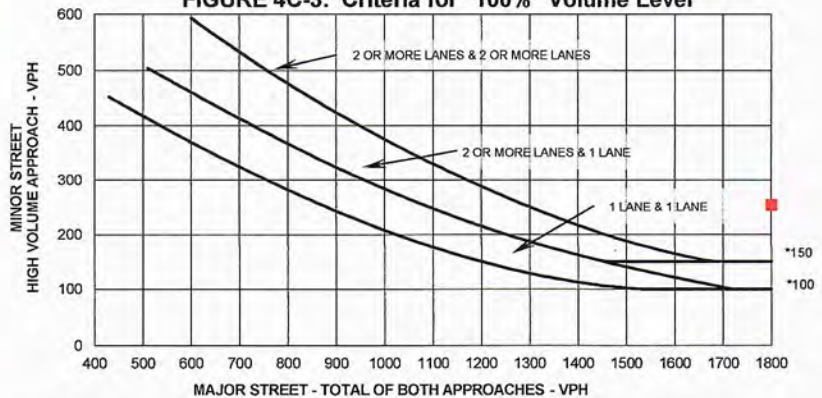
1. Delay on Minor Approach *(vehicle-hours)		
Approach Lanes	1	2
Delay Criteria*	4.0	5.0
Delay*		89.9
Fulfilled?:	☒ Yes	☐ No

2. Volume on Minor Approach One-Direction *(vehicles per hour)		
Approach Lanes	1	2
Volume Criteria*	100	150
Volume*		253
Fulfilled?:	☒ Yes	☐ No

3. Total Intersection Entering Volume *(vehicles per hour)		
No. of Approaches	3	4
Volume Criteria*	650	800
Volume*		2,099
Fulfilled?:	☒ Yes	☐ No

Plot volume combination on the applicable figure below.

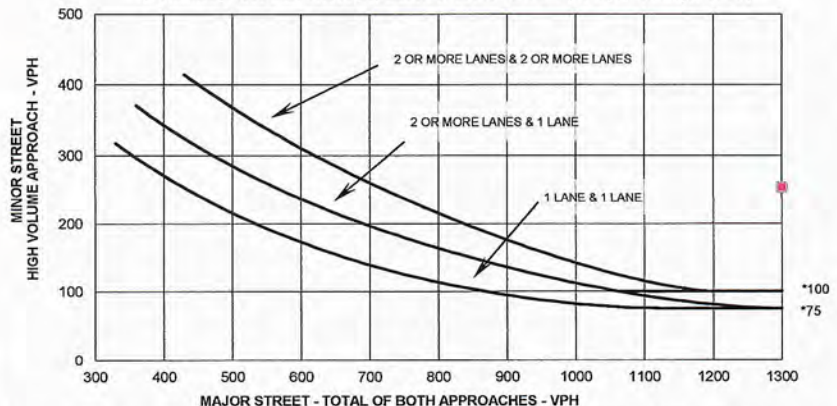
FIGURE 4C-3: Criteria for "100%" Volume Level



* Note: 150 vph applies as the lower threshold volume for a minor street approach with two or more lanes and 100 vph applies as the lower threshold volume threshold for a minor street approach with one lane.

FIGURE 4C-4: Criteria for "70%" Volume Level

(Community Less than 10,000 population or above 70 km/hr (40 mph) on Major Street)



* Note: 100 vph applies as the lower threshold volume for a minor street approach with two or more lanes and 75 vph applies as the lower threshold volume threshold for a minor street approach with one lane.

TURNING MOVEMENT VOLUME COUNTS

N/S STREET: Kanter Hwy
 FILENAME: 3/20/2019
 COUNT DATE: 12/16/2020
 REPORT DATE: 3/20/2019
 DAY: Wednesday
 ANALYSIS YEAR: 2024
 E/W STREET: Driveway 1
 CITY: Stuart
 INTERSECTION: 1185
 CONTROL: 1165
 (Costco Blvd)

15 Min Period	Northbound				Southbound				Eastbound				Westbound				ONE HOUR SUM
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL	WBL	WBT	WBR	
7:00-7:15	0	163	0	0	277	0	0	0	0	0	0	0	440	0	0	0	2040
7:15-7:30	0	230	0	0	206	0	0	0	0	0	0	0	526	0	0	0	2149
7:30-7:45	0	254	0	0	253	0	0	0	0	0	0	0	507	0	0	0	2138
7:45-8:00	0	288	0	0	269	0	0	0	0	0	0	0	567	0	0	0	2127
8:00-8:15	0	284	0	0	265	0	0	0	0	0	0	0	549	0	0	0	1976
8:15-8:30	0	329	0	0	186	0	0	0	0	0	0	0	515	0	0	0	486
8:30-8:45	0	256	0	0	240	0	0	0	0	0	0	0	496	0	0	0	416
8:45-9:00	0	218	0	0	198	0	0	0	0	0	0	0	416	0	0	0	416

AM PEAK HOUR IS FROM: 7:15AM TO 8:15AM
 Volumes: 0 1066 0 0 1083 0 0 0 0 0 0 0 0 2149
 Season Factor: 0 1066 0 0 1083 0 0 0 0 0 0 0 0 2149
 Growth: 0 1093 0 0 1110 0 0 0 0 0 0 0 0 2203
 In/Out: - IN/OUT IN IN - - - - - OUT - OUT -
 Res Percentage: 0% 0% 20% 15% 0% 0% 0% 0% 0% 0% 0% 0% 0% 15% 0% 0% 0% 15% 0%
 Retail Percentage: 0% 5% 10% 20% 30% 0% 0% 0% 0% 0% 0% 0% 0% 35% 0% 0% 0% 15% 0%
 PROJECT: 0 31 53 75 0 0 0 0 0 0 0 0 0 83 0 0 0 41 0
 MMHS:

Total	0	1124	53	75	1110	0	0	0	0	0	90	0	41	2403				
-------	---	------	----	----	------	---	---	---	---	---	----	---	----	------	--	--	--	--

15 Min Period	Northbound				Southbound				Eastbound				Westbound				ONE HOUR SUM
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL	WBL	WBT	WBR	
4:00-4:15	0	317	0	0	226	0	0	0	0	0	0	0	543	0	0	0	2010
4:15-4:30	0	281	0	0	209	0	0	0	0	0	0	0	484	0	0	0	1993
4:30-4:45	0	287	0	0	212	0	0	0	0	0	0	0	499	0	0	0	1984
4:45-5:00	0	240	0	0	244	0	0	0	0	0	0	0	484	0	0	0	1978
5:00-5:15	0	275	0	0	251	0	0	0	0	0	0	0	526	0	0	0	2001
5:15-5:30	0	260	0	0	215	0	0	0	0	0	0	0	475	0	0	0	1984
5:30-5:45	0	271	0	0	222	0	0	0	0	0	0	0	489	0	0	0	1993
5:45-6:00	0	292	0	0	215	0	0	0	0	0	0	0	507	0	0	0	2010

PM PEAK HOUR IS FROM: 4:00PM TO 5:00PM
 Volumes: 0 1125 0 0 885 0 0 0 0 0 0 0 0 2010
 Season Factor: 0 1125 0 0 885 0 0 0 0 0 0 0 0 2010
 Growth: 0 1153 0 0 907 0 0 0 0 0 0 0 0 2061
 In/Out: - IN/OUT IN IN - - - - - OUT - OUT -
 Res Percentage: 0% 0% 20% 15% 0% 0% 0% 0% 0% 0% 0% 0% 0% 15% 0% 0% 0% 15% 0%
 Retail Percentage: 0% 5% 10% 20% 30% 0% 0% 0% 0% 0% 0% 0% 0% 35% 0% 0% 0% 15% 0%
 PROJECT: 0 69 113 158 0 0 0 0 0 0 0 0 0 165 0 0 0 74 0
 MMHS:

Total	0	1222	113	158	907	0	0	0	0	0	203	0	74	2677				
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HCS7 Two-Way Stop-Control Report

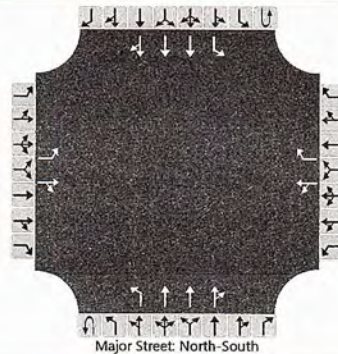
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	11/30/2020
Analysis Year	2020
Time Analyzed	PM Peak Hour
Intersection Orientation	North-South
Project Description	M & M Retail

Site Information

Intersection	Kanner Hwy and Driveway 1
Jurisdiction	City of Stuart
East/West Street	Driveway 1
North/South Street	Kanner Hwy
Peak Hour Factor	0.92
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	1	0		0	1	1	0	1	3	0	0	1	3	0
Configuration		L		TR		LT		R		L	T	TR		L	T	TR
Volume (veh/h)		2	1	2		203	1	74	0	1	1222	113	0	158	907	1
Percent Heavy Vehicles (%)		3	3	3		0	3	0	3	3			3	3		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized					No											
Median Type Storage	Left + Thru								1							

Critical and Follow-up Headways

Base Critical Headway (sec)		6.4	6.5	7.1		6.4	6.5	7.1		5.3				5.3		
Critical Headway (sec)		6.46	6.56	7.16		6.40	6.56	7.10		5.36				5.36		
Base Follow-Up Headway (sec)		3.8	4.0	3.9		3.8	4.0	3.9		3.1				3.1		
Follow-Up Headway (sec)		3.83	4.03	3.93		3.80	4.03	3.90		3.13				3.13		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		2		3		222		80		1				172		
Capacity, c (veh/h)		28		444		53		319		394				233		
v/c Ratio		0.08		0.01		4.16		0.25		0.00				0.74		
95% Queue Length, Q ₉₅ (veh)		0.2		0.0		24.5		1.0		0.0				5.1		
Control Delay (s/veh)		146.8		13.2		1579.3		20.1		14.2				54.0		
Level of Service (LOS)		F		B		F		C		B				F		
Approach Delay (s/veh)	66.6				1164.3				0.0				8.0			
Approach LOS	F				F											



O'ROURKE
ENGINEERING & PLANNING

TRAFFIC ANALYSIS

FOR

KANNER CPUD

Prepared for:

Mr. Joe Marino
M & M Retail Partners, LLC.
Lodi, NJ 07644
973-779-7466

Prepared by

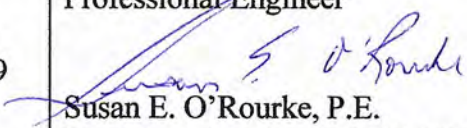
O'Rourke Engineering & Planning
22 SE Seminole Street
Stuart, Florida 34994
772-781-7918

December 16, 2020
Revised March 16, 2021
Revised April 21, 2021

MR18031.0

Prepared by:
O'Rourke Engineering & Planning
Certificate of Authorization: #26869
22 SE Seminole Street
Stuart, Florida 34994
(772) 781-7918

Professional Engineer


Susan E. O'Rourke, P.E.

Date signed and sealed: 4/21/2021

License #: 42684



O'ROURKE
ENGINEERING & PLANNING

December 16, 2020

Mr. Joe Marino
M & M Retail Partners, LLC.
Lodi, NJ 07644

Re: Kanner CPUD

Dear Mr. Marino:

O'Rourke Engineering & Planning has completed the traffic analysis for the proposed development including 157,531 square feet of Discount Club, 16,750 square feet of Shopping Center, 16,240 square feet of restaurant, 2,500 square feet of fast food with drive thru, and 398 units of Multifamily Housing (Mid-Rise) to be located east of Kanner Highway and approximately 2,000 feet north of Indian Street in Stuart, FL. The steps in the analysis and the ensuing results are presented herein.

It has been a pleasure working with you. If you have any questions or comments, please give me a call.

Respectfully submitted,
O'ROURKE ENGINEERING & PLANNING

Susan E. O'Rourke, P.E.
Registered Civil Engineer - Traffic

C6 – Traffic Analysis Report – M&M Retail – 4.21.21

TABLE OF CONTENTS

INTRODUCTION	1
PROJECT DESCRIPTION	1
EXISTING CONDITIONS	1
PROJECT TRAFFIC	1
PROJECT DISTRIBUTION/ ASSIGNMENT/ IMPACT	4
FUTURE TRAFFIC	4
INTERSECTIONS / DRIVEWAYS	9
SIGNAL WARRANT DATA AND OTHER	9
CONCURRENCY REVIEW/ CONCLUSION	14

TABLES

Table 1: Trip Generation	3
Table 2: Project Percent Impact	7
Table 3: Link Analysis	8
Table 4: Right Turn Lane Analysis	13
Table 5: Intersection Results Summary	14
Table 6: Turn Lane Queue Analysis	14

FIGURES

Figure 1: Project Location	2
Figure 2a: Retail Percent Assignment	5
Figure 2b: Residential Percent Assignment	6
Figure 3a: Driveway Assignment - Retail	10
Figure 3b: Driveway Assignment – Residential	11
Figure 3c: Total Driveway Volumes	12

APPENDICES

APPENDIX A: Site Plan	
APPENDIX B: Non-Motorized Features	
APPENDIX C: Internal Capture	
APPENDIX D: Martin County 2019 Roadway Level of Service Inventory Report	
APPENDIX E: Driveway Data and Analyses	

INTRODUCTION

O'Rourke Engineering & Planning was retained to prepare a traffic analysis for the proposed development including 157,531 square feet of Discount Club, 16,750 square feet of Shopping Center, 16,240 square feet of restaurant, 2,500 square feet of fast food with drive thru, and 398 units of Multifamily Housing (Mid-Rise) to be located east of Kanner Highway and approximately 2,000 feet north on Indian Street in Stuart, Florida. The purpose of this analysis is to determine the impact of the development on the roadway.

PROJECT DESCRIPTION

The approximately 48.97-acre parcel is proposed to be developed with 157,531 square feet of Discount Club, 16,750 square feet of Shopping Center, 16,240 square feet of restaurant, 2,500 square feet of fast food with drive thru, and 398 units of Multifamily Housing (Mid-Rise). **Figure 1** shows the project location. The project site plan is included in **Appendix A**.

EXISTING CONDITIONS

The study area is defined as the roadways upon which the project has an impact of 5% of the level of service capacity of the roadway. Once the project traffic was assigned, the study area was refined based on the impact percentages.

The study area roadways were defined in terms of existing lane geometrics and existing traffic volumes. The non-motorized transportation components are included in **Appendix B**.

Existing Lane Geometrics

The study area was reviewed to determine the existing number and type of lanes and the traffic control along the roadway. Each roadway is described below.

Kanner Highway is a six-lane divided arterial with a north/south alignment.

Willoughby Boulevard is a four-lane divided arterial with a north/south alignment.

Indian Street is a four-lane divided collector with an east/west alignment.

Monterey Road is a four-lane divided arterial with an east/west alignment.

PROJECT TRAFFIC

To estimate traffic generated by the land uses defined in the scope of this project, the ITE Trip Generation, 10th Edition trip rates for land use codes 221-Multifamily Housing (Mid-Rise), 820-Retail Shopping Center, 932-High-Turnover (Sit-Down) Restaurant, 934 Fast Food (w/ Drive-Thru), and 857-Discount Club were applied. These calculations provide an estimate of the typical generation for the proposed use. Trip generation for the project in the Daily, AM and PM Peak Hours is shown in **Table 1**. The details of the internal capture and pass-by are included in **Appendix C**.

As shown, the project will generate 7,410 daily trips, 341 in the AM peak hour, and 668 in the PM peak hour.



Table 1 - Trip Generation

Table 1a: Daily

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips			Net External Trips			Pass-by Trips			Net New Trips				
					In	Out	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Multifamily Housing (Mid-Rise)	221	398	DU	$T = 5.65(X) - 1.75$	50%	50%	1,084	1,083	2,167	76	228	304	14.0%	1,008	855	1,863	-	-	-	1,008	855	1,863
Shopping Center	820	16,750	SR	$\ln(T) = 0.681 \ln(X) + 5.57$	50%	50%	892	892	1,784	47	121	168	9.4%	845	771	1,616	388	388	776	457	387	840
Discount Club	857	157,531	SR	$T = 41.80(X)$	50%	50%	3,293	3,292	6,585	174	445	619	9.4%	3,119	2,847	5,966	1,163	1,163	2,326	1,956	1,884	3,640
Fast Food (Drive Thru)	934	2,500	SR	$T = 470.95(X)$	50%	50%	589	588	1,177	299	104	403	34.2%	290	484	774	190	189	379	100	295	395
High-Turnover (Sit-Down) Restaurant	932	16,240	SR	$T = 112.18(X)$	50%	50%	911	911	1,822	462	160	622	34.1%	449	751	1,200	264	264	528	487	672	1,159
							5,685	5,683	11,368	982	830	1,812	15.9%	4,703	4,853	9,556	2,004	4,009	42.0%	2,698	2,849	5,547
Retail Sub Total							6,769	6,766	13,535	1,058	1,058	2,116	15.6%	5,711	5,708	11,419	2,005	4,009	42.0%	3,706	3,704	7,410
TOTALS		193,021																				

Sources: ITE 10th Edition Trip Generation Rates
Total 162,020 w/ OutdoorVestibule 4,489

Table 1b: AM Peak Hour

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips			Net External Trips			Pass-by Trips			Net New Trips					
					In	Out	In	Out	Total	In	Out	Total	%	In	Out	Total	In	Out	Total	%	In	Out	Total
Multifamily Housing (Mid-Rise) Shopping Center Discount Club Fast Food (w/ Drive Thru) High-Turnover (Sit-Down) Restaurant Retail Sub Total	221	398	DJ	$\text{Ln}(T) = 0.981 \ln(X) - 0.98$	26%	74%	35	98	133	3	21	24	18.0%	32	77	109	-	-	-	0.0%	32	77	109
	820	16,750	SR	$T = 0.50(X) + 151.78$	62%	38%	99	61	160	9	8	17	10.6%	90	53	143	35	34	69	48.0%	55	19	74
	857	157,531	SR	$T = 0.49(X)$	70%	30%	54	23	77	4	8	10.4%	50	19	69	14	13	27	39.0%	36	6	42	
	934	2,500	SR	$T = 40.19(X)$	51%	49%	51	49	100	12	5	17	17.0%	39	44	83	21	20	41	49.0%	18	24	42
	932	16,240	SR	$T = 9.94(X)$	55%	45%	89	72	161	19	9	28	17.4%	70	63	133	30	29	59	44.0%	40	34	74
							293	205	498	44	26	70	14.1%	249	179	428	100	96	196	45.8%	149	83	232
TOTALS		193,021				328	303	631	47	94	141%	281	256	537	100	96	196	36.5%	181	160	341		

Sources: ITE 10th Edition Trip Generation Rates

Table 1c: PM Peak Hour

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips			Net External Trips			Pass-by Trips			Net New Trips		
					In	Out	In	Out	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total
Multifamily Housing (Mid-Rise)	221	398	DJ	$\text{Ln}(T) = 0.861 \ln(X) - 0.63$	61%	39%	102	65	167	25	20	45	26.9%	77	45	122	-	-	-	0.0%
Shopping Center	820	16,750	SR	$\text{Ln}(T) = 0.741 \ln(X) + 2.89$	48%	52%	70	75	145	4	7	11	7.6%	66	134	200	32	33	65	48.0%
Discount Club	857	157,531	SR	$T = 4.18(X)$	50%	50%	329	329	658	19	29	48	7.3%	310	300	610	119	119	238	39.0%
Fast Food (w/ Drive Thru)	934	2,500	SR	$T = 32.67(X)$	52%	48%	43	39	82	10	7	17	20.7%	33	32	65	16	16	32	49.0%
High-Turnover (Sit-Down) Restaurant	932	16,240	SR	$T = 9.77(X)$	62%	38%	99	60	159	18	13	31	19.5%	81	47	128	28	28	56	44.0%
TOTALS		193,021					541	503	1,044	51	56	107	10.2%	490	447	937	195	196	391	41.7%
							643	568	1,211	76	76	152	12.6%	567	492	1,059	195	196	391	36.9%

Sources: ITE 10th Edition Trip Generation Rates

PROJECT DISTRIBUTION/ ASSIGNMENT/ IMPACT

The project traffic was distributed by general geographic direction and then assigned to the roadway network. Two different traffic assignments were made for the retail use and the residential use.

Distribution/Assignment – The distributions led to an assignment of trips based on the anticipated ultimate destinations and the roadway paths used to reach those destinations. **Figure 2a and 2b** shows the project assignment for both retail and residential land uses.

Table 2 summarizes the resultant AM and PM peak hour trips assigned to the network and the project % of capacity.

Impact – As seen in Table 2, there are no links with a 5% or greater impact. Therefore, the directly accessed links on Kanner Highway and Willoughby Boulevard were analyzed.

FUTURE TRAFFIC

Since the project traffic is not significant, the adjacent links were analyzed for future traffic volumes. **Table 3** shows that the total traffic on the adjacent links of Kanner Highway and Willoughby Boulevard will operate at acceptable levels of service at project buildout of 2024. To estimate future 2024 volumes on Kanner Highway and Willoughby Boulevard, the existing (2019) traffic volumes were increased by a growth rate found in the Martin County 2019 Roadway Level of Service Inventory Report. The growth rate was then applied for three years to estimate the 2024 peak hour directional volumes. The project traffic was then added to estimate the 2024 Total Traffic volumes with project traffic added. The total traffic volumes were then compared to the acceptable threshold.

The Martin County 2019 Roadway Level of Service Inventory Report is included in the **Appendix D**.



22 SE Seminole St
Stuart, FL 34994

Date: 5/21/18



NTS

Job Number: MRI8031.0



Table 2a: Percent Impact - AM Peak Hour

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	Retail		Residential		Total Project Traffic	Directional Percent Project of Capacity
						Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume		
Kanner Highway (SR-76)	Salerno Rd	Indian St	NB	6LD	no	15%	22	30%	10	32	1.06%
	Salerno Rd	Indian St	SB	6LD	no	15%	12	30%	23	35	1.16%
	Indian St	Project Entrance	NB	6LD	no	35%	52	25%	8	60	1.99%
	Indian St	Project Entrance	SB	6LD	no	35%	29	25%	19	48	1.59%
	Project Entrance	SR-714	NB	6LD	no	40%	33	20%	15	48	1.59%
	Project Entrance	SR-714	SB	6LD	no	40%	60	20%	6	66	2.19%
	SR-714	SR-5	NB	6LD	no	20%	17	30%	23	40	1.32%
	SR-714	SR-5	SB	6LD	no	20%	30	30%	10	40	1.32%
	Willoughby Blvd	Pomeroy St	NB	2L	no	5%	7	5%	2	9	1.02%
	Salerno Rd	Pomeroy St	SB	2L	no	5%	4	5%	4	8	0.91%
SR-714 (Monterey Rd)	Salerno Rd	Indian St	NB	4LD	no	5%	7	5%	2	9	0.45%
	Pomeroy St	Indian St	SB	4LD	no	5%	4	5%	4	8	0.40%
	Pomeroy St	Project Entrance	NB	4LD	no	5%	7	25%	8	15	0.75%
	Indian St	Project Entrance	SB	4LD	no	10%	8	25%	19	27	1.35%
	Project Entrance	SR-714	NB	4LD	no	15%	12	30%	23	35	1.75%
	Project Entrance	SR-714	SB	4LD	no	15%	22	30%	10	32	1.60%
	SR-76	Willoughby Blvd	EB	4LD	no	0%	0	10%	8	8	0.40%
	SR-76	Willoughby Blvd	WB	4LD	no	0%	0	10%	3	3	0.15%
	SR-76	Willoughby Blvd	EB	4LD	no	0%	0	15%	12	12	0.60%
	SR-76	Willoughby Blvd	WB	4LD	no	5%	7	15%	5	12	0.60%
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	EB	4LD	no	20%	30	10%	3	33	1.65%
	Mapp Rd	SR-76	WB	4LD	no	20%	17	10%	8	25	1.25%

Source: Martin County 2019 Roadway Level of Service Inventory Report

Retail	IN	OUT
Residential	149	83
	32	77

Table 2b Percent Impact - PM Peak Hour

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	Retail		Residential		Total Project Traffic	Directional Percent Project of Capacity
						Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume		
Kanner Highway (SR-76)	Salerno Rd	Indian St	NB	6LD	no	15%	44	30%	23	67	2.22%
	Salerno Rd	Indian St	SB	6LD	no	15%	38	30%	14	52	1.72%
	Indian St	Project Entrance	NB	6LD	no	35%	103	25%	19	122	4.04%
	Indian St	Project Entrance	SB	6LD	no	35%	88	25%	11	99	3.28%
	Project Entrance	SR-714	NB	6LD	no	40%	100	20%	9	109	3.61%
	Project Entrance	SR-714	SB	6LD	no	40%	118	20%	15	133	4.40%
	SR-714	SR-5	NB	6LD	no	20%	50	30%	14	64	2.12%
	SR-714	SR-5	SB	6LD	no	20%	59	30%	23	82	2.72%
	Willoughby Blvd	Pomeroy St	NB	2L	no	5%	7	5%	4	11	1.25%
	Salerno Rd	Pomeroy St	SB	2L	no	5%	13	5%	2	15	1.70%
SR-714 (Monterey Rd)	Salerno Rd	Indian St	NB	4LD	no	5%	7	5%	4	11	0.55%
	Pomeroy St	Indian St	SB	4LD	no	5%	4	5%	2	6	0.30%
	Pomeroy St	Project Entrance	NB	4LD	no	5%	7	25%	19	26	1.30%
	Indian St	Project Entrance	SB	4LD	no	10%	8	25%	11	19	0.95%
	Project Entrance	SR-714	NB	4LD	no	15%	12	30%	14	26	1.30%
	Project Entrance	SR-714	SB	4LD	no	15%	22	30%	23	45	2.25%
	SR-76	Willoughby Blvd	EB	4LD	no	0%	0	15%	7	7	0.35%
	SR-76	Willoughby Blvd	WB	4LD	no	0%	0	15%	12	12	0.60%
	SR-76	Willoughby Blvd	EB	4LD	no	0%	0	15%	7	7	0.35%
	SR-76	Willoughby Blvd	WB	4LD	no	5%	15	15%	12	27	1.35%
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	EB	4LD	no	20%	59	10%	8	67	3.35%
	Mapp Rd	SR-76	WB	4LD	no	20%	50	10%	5	55	2.75%

Source: Martin County 2019 Roadway Level of Service Inventory Report

Retail	IN	OUT
Residential	295	251
	77	45

Table 3a: Link Analysis - AM Peak Hour

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	2019 ADOT D Factor (1)	2019 Volume Peak Hour Peak Direction	Growth Rate	2024 Volume Peak Hour Direction	LOS D Directional Peak Hour	Retail			Residential			Total Traffic (Peak Direction)	Does Project Meet Concurrency?
											Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume		
Kanner Highway (SR-76)	Indian St	Project Entrance	NB	IN	6LD	no	21,116	1.005	613	3020	35%	52	25%	8	60	1.99%	673	Yes
	Indian St	Project Entrance	SB	OUT	6LD	no	21,116	1.005	1018	3020	35%	29	25%	19	48	1.59%	1066	Yes
	Project Entrance	SR-714	NB	OUT	6LD	no	21,116	1.005	613	3020	40%	33	20%	15	48	1.59%	661	Yes
	Project Entrance	SR-714	SB	IN	6LD	no	21,116	1.005	1018	3020	40%	60	20%	6	66	2.19%	1084	Yes
Willoughby Blvd	Indian St	Project Entrance	NB	IN	4LD	no	11,100	1.037	481	2000	5%	8	25%	8	16	0.75%	497	Yes
	Indian St	Project Entrance	SB	OUT	4LD	no	11,100	1.037	799	2000	10%	12	25%	19	31	1.35%	830	Yes
	Project Entrance	SR-714	NB	OUT	4LD	no	11,100	1.037	481	2000	15%	12	30%	23	35	1.75%	516	Yes
	Project Entrance	SR-714	SB	IN	4LD	no	11,100	1.037	799	2000	15%	22	30%	10	32	1.60%	831	Yes

Source: Martin County 2019 Roadway Level of Service Inventory Report

(1) FDOT Florida Traffic Online

Retail		IN	OUT
Residential		149	83
		32	77

Years Growth: 5

Table 3b: Link Analysis - PM Peak Hour

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	2019 ADOT D Factor (1)	2019 Volume Peak Hour Peak Direction	Growth Rate	2024 Volume Peak Hour Direction	LOS D Directional Peak Hour	Retail			Residential			Total Traffic (Peak Direction)	Does Project Meet Concurrency?
											Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume		
Kanner Highway (SR-76)	Indian St	Project Entrance	NB	IN	6LD	no	21,116	1.005	1018	3020	35%	103	25%	19	122	4.04%	1140	Yes
	Indian St	Project Entrance	SB	OUT	6LD	no	21,116	1.005	613	3020	35%	88	25%	11	99	3.28%	712	Yes
	Project Entrance	SR-714	NB	OUT	6LD	no	21,116	1.005	1018	3020	40%	100	20%	9	109	3.61%	1127	Yes
	Project Entrance	SR-714	SB	IN	6LD	no	21,116	1.005	613	3020	40%	118	20%	15	133	4.40%	746	Yes
Willoughby Blvd	Indian St	Project Entrance	NB	IN	4LD	no	11,100	1.037	799	2000	5%	7	25%	19	26	1.30%	825	Yes
	Indian St	Project Entrance	SB	OUT	4LD	no	11,100	1.037	481	2000	10%	8	25%	11	19	0.95%	500	Yes
	Project Entrance	SR-714	NB	OUT	4LD	no	11,100	1.037	799	2000	15%	12	30%	14	26	1.30%	825	Yes
	Project Entrance	SR-714	SB	IN	4LD	no	11,100	1.037	481	2000	15%	22	30%	23	45	2.25%	526	Yes

Source: Martin County 2019 Roadway Level of Service Inventory Report

(1) FDOT Florida Traffic Online

Retail		IN	OUT
Residential		295	251
		77	45

Years Growth: 5

INTERSECTIONS / DRIVEWAYS

The intersection analysis was limited to the intersections of the driveways with Kanner and the driveway with Willoughby. Since the project is not significant on any link, additional intersection analysis was not required.

The project has proposed four driveways.

- Driveway 1 will be a full access signalized intersection to Kanner Highway with two westbound left-turn lanes leaving the project.
- Driveway 2 will have access to Kanner Highway and will be a directional median with a left-in, right-in, right-out only.
- Driveway 3 will have full access onto Willoughby Boulevard.
- Driveway 4 will be a right-in/right-out driveway on Kanner Highway.

The project will also have a cross-access to Indian Street and other driveways on Kanner Highway. For purposes of this study, we have shown traffic at project driveways 1, 2, 3, and 4 and at the cross-access driveway on Indian Street.

Figures 3a and 3b show the driveway percent assignments for the retail and residential portions, respectively. **Figure 3c** illustrates the total of the retail and residential project volumes in the AM and PM peak hour. The proposed driveway lanes are summarized below:

Driveway 1/ Kanner Highway (Full Access) (The lane call out is subject to FDOT permitting)

Northbound

- 1 – U-turn lane
- 3 - through lanes
- 1 – right turn lane

Southbound

- 1 – left turn lane
- 2 – through lanes
- 1 – through/right lane

Eastbound

- 1 – left turn lane
- 1 – one through/right lane

Westbound

- 1 – right turn lane
- 2 – left turn lanes

Driveway 2/ Kanner Highway (Directional Access)

Northbound

- 1 – U-turn lane
- 3 – through lanes
- 1 – right turn lane

Southbound

- 1 – left turn lane
- 3 – through lanes

Westbound

- 1 – right turn

Driveway 3/ Willoughby Boulevard (Full Access)

Northbound

- 1 – left turn lane
- 2 – through lanes

Southbound

- 1 – U-turn lane
- 2 – through lanes
- 1 – right turn lane

Eastbound

- 1 – left turn lane
- 1 – left turn lane
- 1 – right turn lane

Westbound

- Future Sailfish Access

Driveway 4/ Kanner Highway (Right-in/Right out)

Northbound

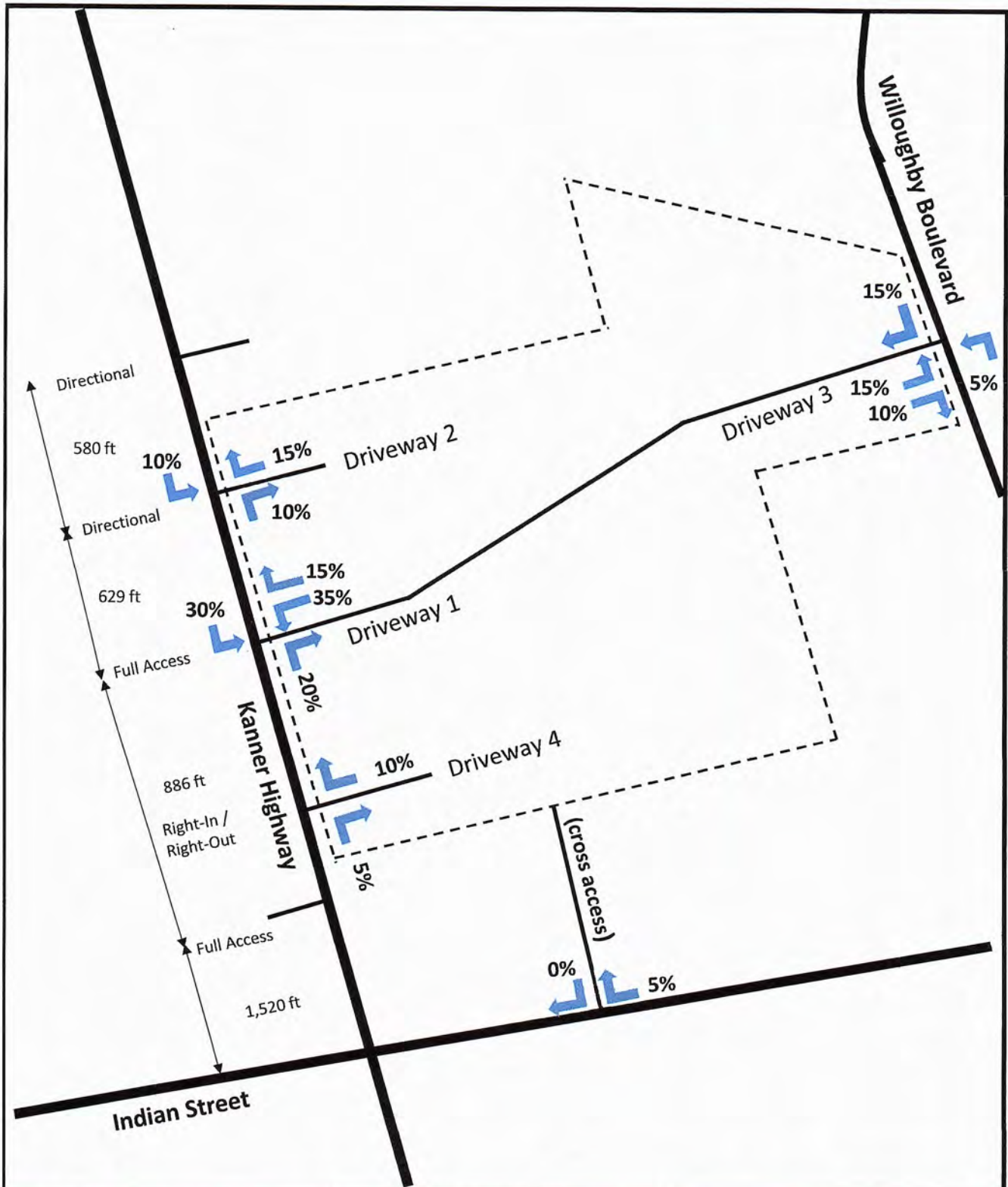
- 2 – through lanes
- 1 – through/ right turn lane

Southbound

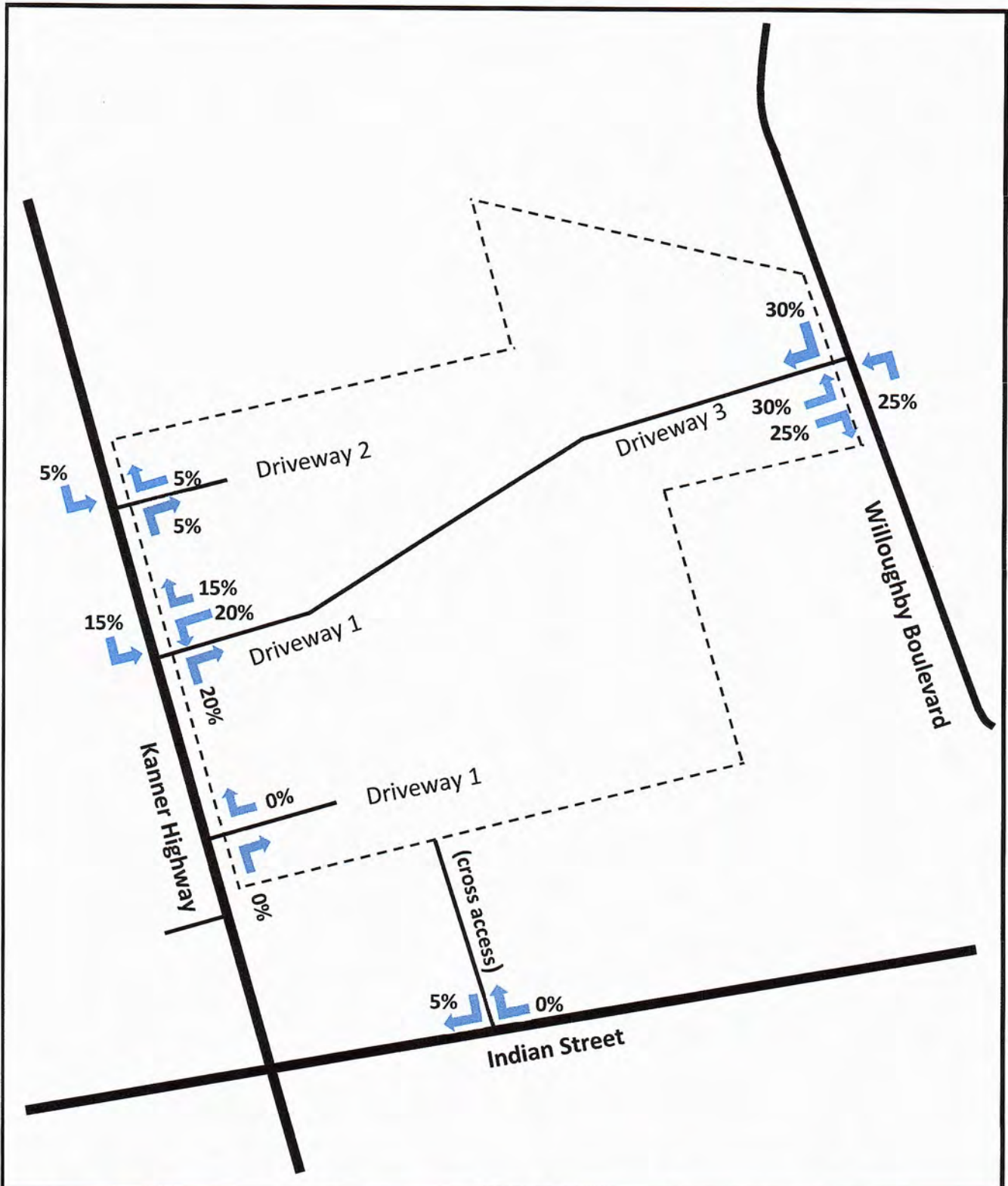
- 3 – through lanes

Westbound

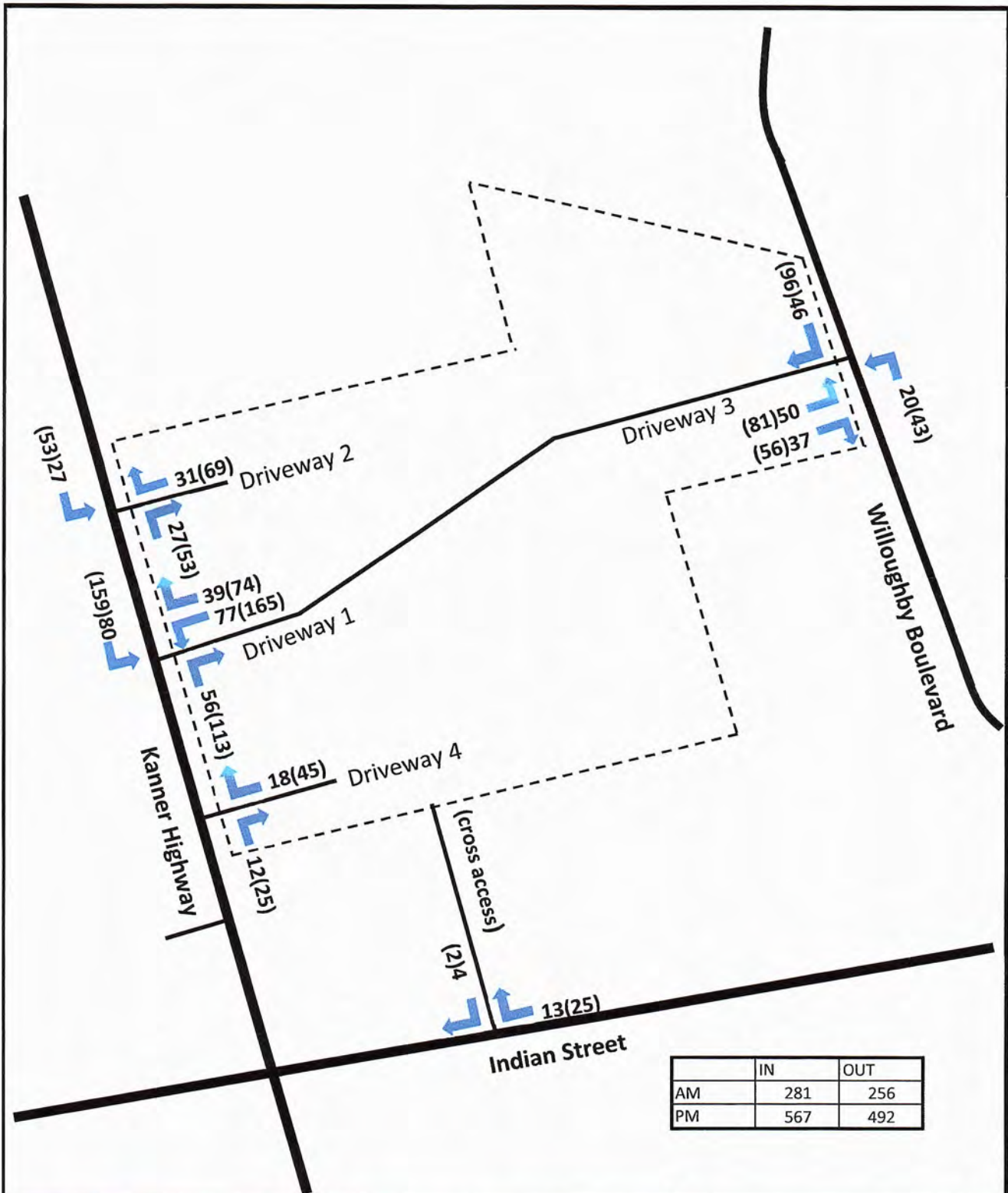
- 1 – right turn lane



  NTS 22 SE Seminole St Stuart, FL, 34994 Job #: MR18031.0 Date: 2/6/20	Legend  = Project Location  = Direction 15% = Percent Project Traffic xx ft. = Driveway Spacing	Figure 3a Driveway Assignment - Retail M & M Retail
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  NTS 22 SE Seminole St Stuart, FL, 34994 Job #: MR18031.0 Date: 3/12/21	Legend  = Project Location  = Direction 15% = Percent Project Traffic	Figure 3b Driveway Assignment - Residential M & M Retail
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NTS 22 SE Seminole St
Stuart, FL, 34994

Job #: MR18031.0 Date: 4/21/21

Legend

XX(XX) = AM(PM)
 = Project Location
 = Direction
15% = Percent Project Traffic

Figure 3c

Total Driveway Volumes

M & M Retail

As shown, ingress turn lanes are proposed for driveways 1, 2, and 3.

A turn lane analysis was conducted for driveway 4 using the FDOT Access Management Guidebook November 2019. For roadways with a posted speed limit of 45 MPH or less, the lower threshold of 80 right-turn vehicles per hour would be most used for higher volume (greater than 600 vehicles per hour, per lane in one direction on the major roadway) or two-lane roads where lateral movement is restricted. The 125 right-turn vehicles per hour upper threshold would be most appropriate on lower volume roadways, multilane highways, or driveways with a large entry radius (50 feet or greater).

Table 4: Right Turn Lane Analysis - Driveway 4

Major Road Speed Limit (MPH)	Number of Lanes (Directional)	Thru Volume (Veh/hr)	Thru Volume (Veh/hr/ln)	Right Turn Volume	Threshold Volume	Right Turn Warranted?
45	3	1018	339	25	125	No

Roadway Posted Speed Limit	Number of Right Turns Per Hour ⁽¹⁾
45 MPH or Less	80-125
Over 45 MPH	35-55

(1) FDOT Access Management Guidebook November 2019, Table 27

As shown, an ingress right-turn lane is not warranted at driveway 4.

The three driveways on Kanner Highway and the driveway on Willoughby Boulevard were analyzed using HCS.

Existing through volumes on Kanner Highway were obtained from the 2019 FDOT 24-hour count on Kanner Highway and the peak hour volumes were calculated and grown to 2024 using the growth rate contained in the Martin County 2019 Roadway Level of Service Inventory Report. Project traffic was then added to obtain the total traffic at the driveways.

The existing peak hour peak direction volume for Willoughby Boulevard was taken from the Martin County 2019 Roadway Level of Service Inventory Report, the FDOT D Factor was then used to convert the existing peak hour peak direction volume into the peak hour off-peak direction volume. These volumes were then grown to 2024 using the growth rate contained in the Martin County 2019 Roadway Level of Service Inventory Report and project traffic was then added.

Table 5 summarizes the results of the driveway analysis. As shown, all driveways will operate at acceptable levels of service at project buildout. **Table 6** summarizes the turn lane lengths and the projected queue for each turn lane.

Table 5: Driveway Results Summary

Intersection	AM Peak Hour			PM Peak Hour		
	Delay	V/C	LOS	Delay	V/C	LOS
Driveway 1 (Costco Blvd) & Kanner Hwy	26.0	0.36	C	38.9	0.38	D
Driveway 2 & Kanner Hwy	9.6	0.04	A	9.9	0.06	A
Driveway 3 & Willoughby Blvd	16.7	0.05	C	15.5	0.08	C
Driveway 4 & Kanner Hwy	14.0	0.05	B	15.9	0.13	C

Table 6: Turn Lane Queue Analysis

Turn Lane Direction	Turn Lane Length	Queue Length
Driveway 1 (Costco Blvd)		
NBR	295	64.9
NBL	345	25 ⁽¹⁾
SBL	345	98.5
Driveway 2		
NBR	275	0
NBL	265	25 ⁽¹⁾
SBL	290	25 ⁽¹⁾
Driveway 3		
NBL	265	25 ⁽¹⁾
SBR	240	0

(1) Less than 1 vehicle queue, 25' used instead

(2) Based on higher of AM or PM queue

The driveway HCS, the retail and residential driveway volumes, and existing FDOT data are included in **Appendix E**.

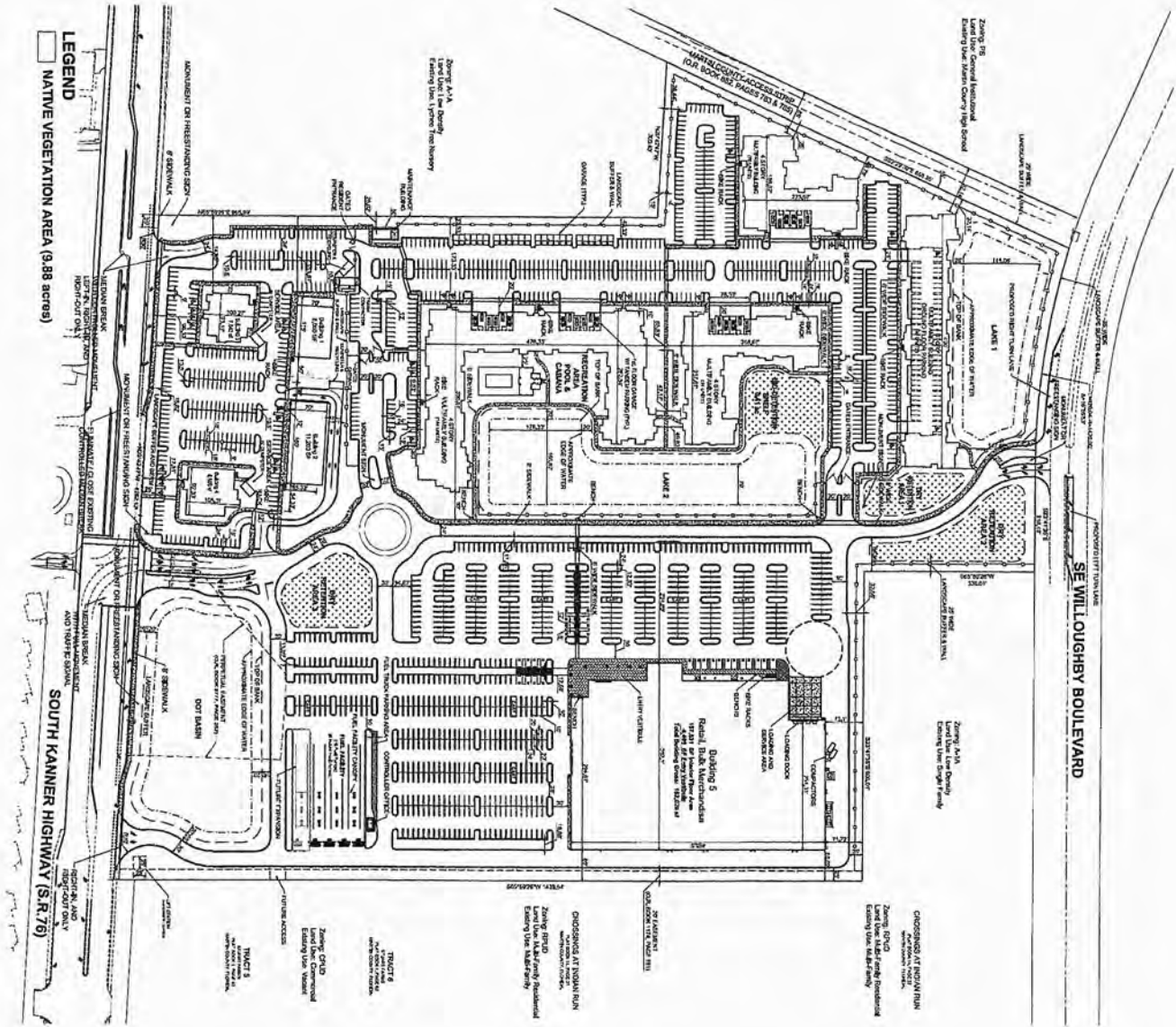
A pre-app meeting was held with FDOT to discuss the driveways. The results of the pre-app will be included in Appendix E when the FDOT finalizes their review. A signal warrant analysis will be required when the applicant applies for a construction permit. Based on the projected volumes at the full-access driveway, it is anticipated that a signal will be warranted. The full warrant analysis was provided to the FDOT under separate cover.

CONCURRENCY REVIEW/ CONCLUSION

The 157,531 square feet of Discount Club, 16,750 square feet of Shopping Center, 16,240 square feet of restaurant, 2,500 square feet of fast food with drive thru, and 398 units of apartment will have a traffic impact of 7,410 Daily trips, 341 AM peak hour trips and 668 PM peak hour trips. With the projects proposed signalization, turn lanes, and inclusion of Costco Boulevard, these project trips will be mitigated. Based on the data and calculations presented herein, the project does not require additional analysis and is deemed to be consistent with transportation concurrency requirements.

APPENDIX A

SITE PLAN



LEGEND	
	NATIVE VEGETATION AREA (9.88 acres)
	ACCESS
	EASEMENT
	RIGHT-OF-WAY
	ROAD
	WATER BODY
	LANDMARK
	BUILDING
	PARKING
	LANDSCAPING
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	LANDMARK

APPENDIX B

NON-MOTORIZED FEATURES



MARTIN MPO BICYCLE, PEDESTRIAN & TRAILS MASTER PLAN

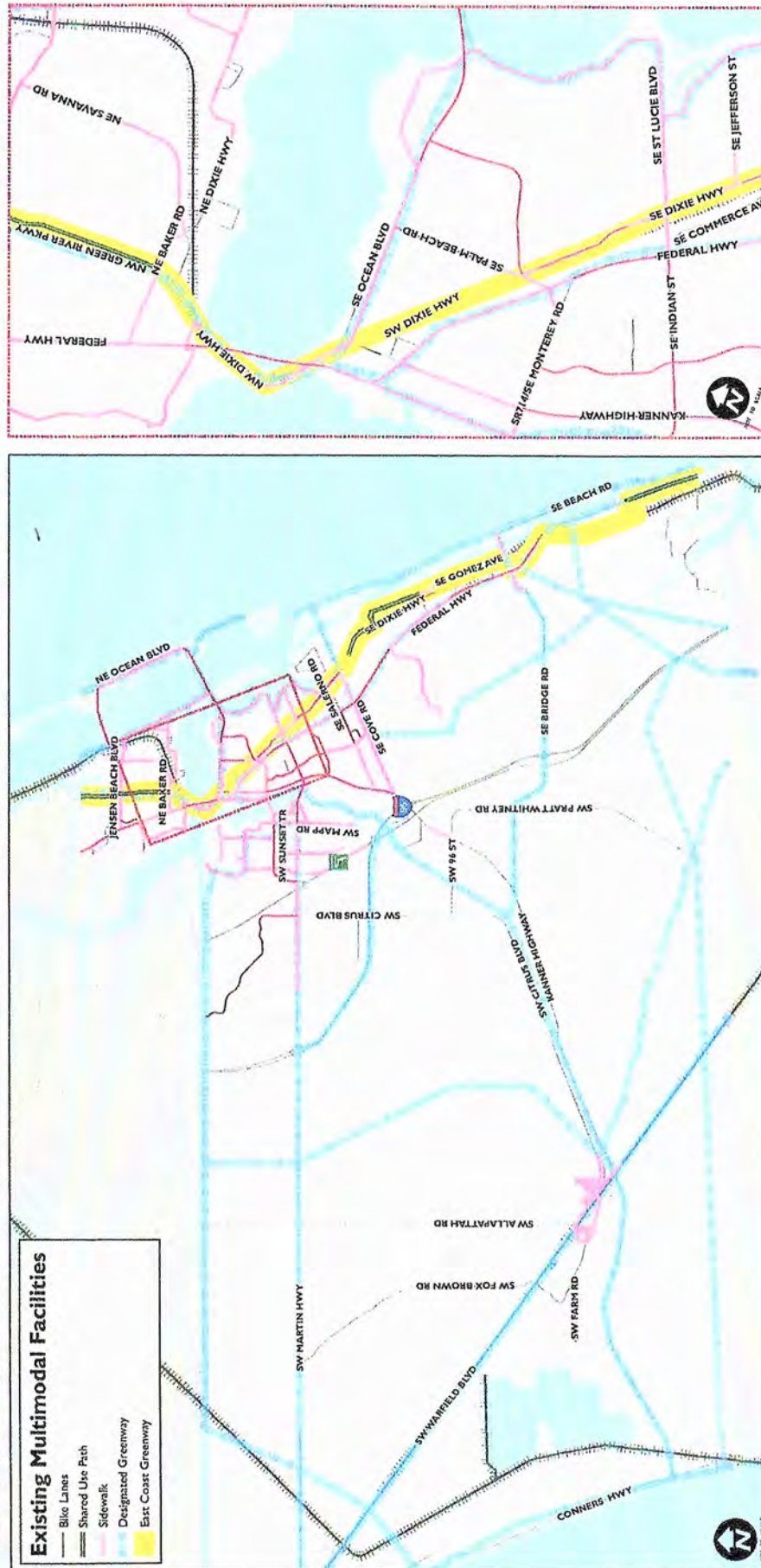


Figure 2-2. Existing Multimodal Facilities

Figure 2-2 provides an overview of existing and designated bicycle and pedestrian facilities. This includes roadways with existing bike lanes, shared use paths, and sidewalks along major roadways. As illustrated, nearly all the existing bike lanes and shared use paths are located within the eastern portion of the County and serve to connect the urbanized areas, included in Figure 2-2 is the proposed East Coast Greenways (ECG) which are trails of regional and statewide significance and connect to Palm Beach and St. Lucie Counties. The light-green network represents the Designated Greenways – these greenways do not necessarily represent existing bike facilities, rather they identify corridors along which to provide a facility.



Figure 2-3 illustrates the locations of the transit routes and stops in Martin County. Four bus routes serve Martin County: Route 1US 1 Corridor, Route 2Indian town, Route 3Stuart, and Route 20X. Route 1US 1 Corridor provides service from Port Salerno north to St. Lucie County while Route 20X provides commuter service south to Palm Beach County. Route 2Indian town serves as a connector between Indian town and the urbanized east coast of Martin County while Route 3Stuart provides a circular service within the Stuart urbanized area.

APPENDIX C
INTERNAL CAPTURE

TABLE 1: Daily Internal Traffic

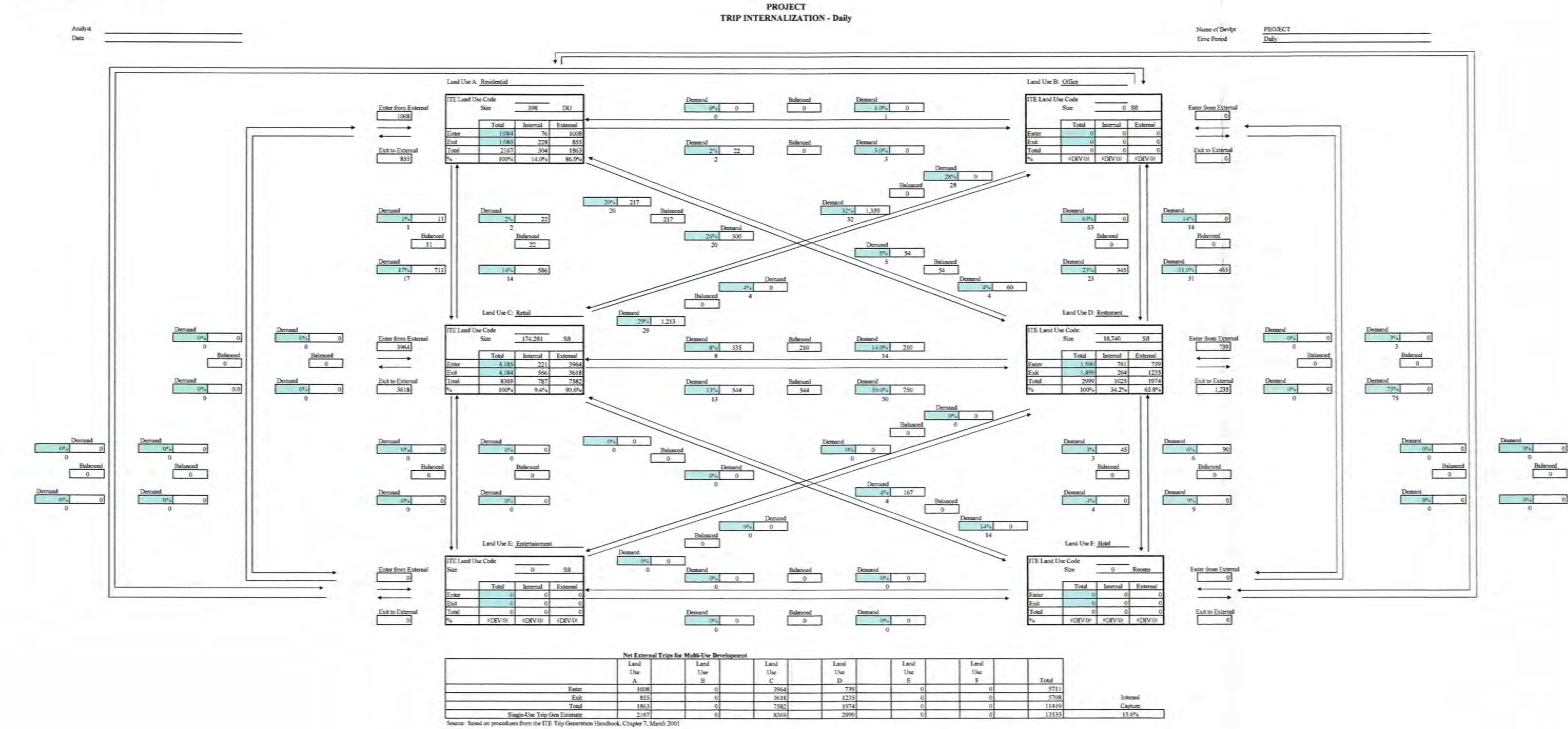
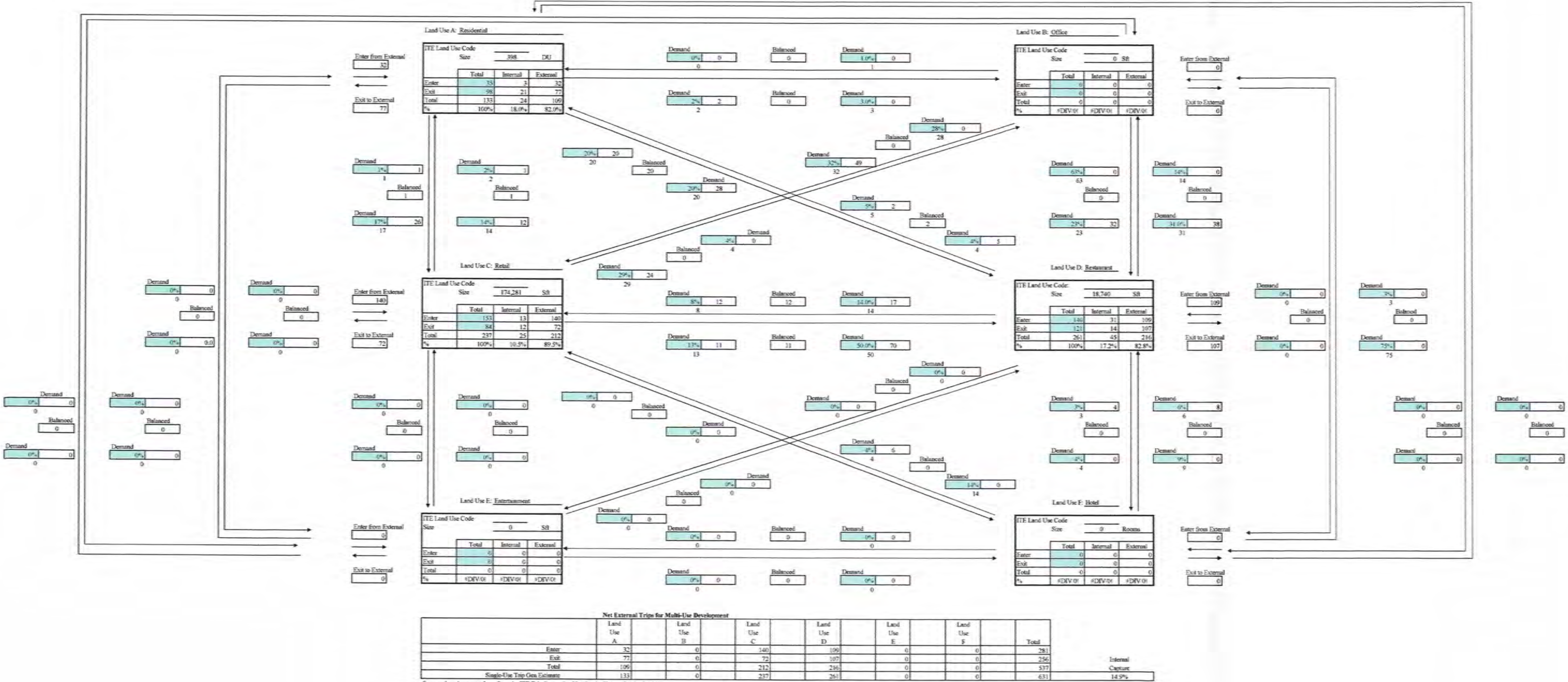


TABLE 1: AM Internal Traffic - Phase I

PROJECT
TRIP INTERNALIZATION - AM

Name of Dev't: PROJECT
Time Period: AM Peak Hour



Note: Numbers outside of the Box are the ITE Pairing Values. Numbers inside the Box are Actual Values used.

TABLE 1: PM Internal Traffic

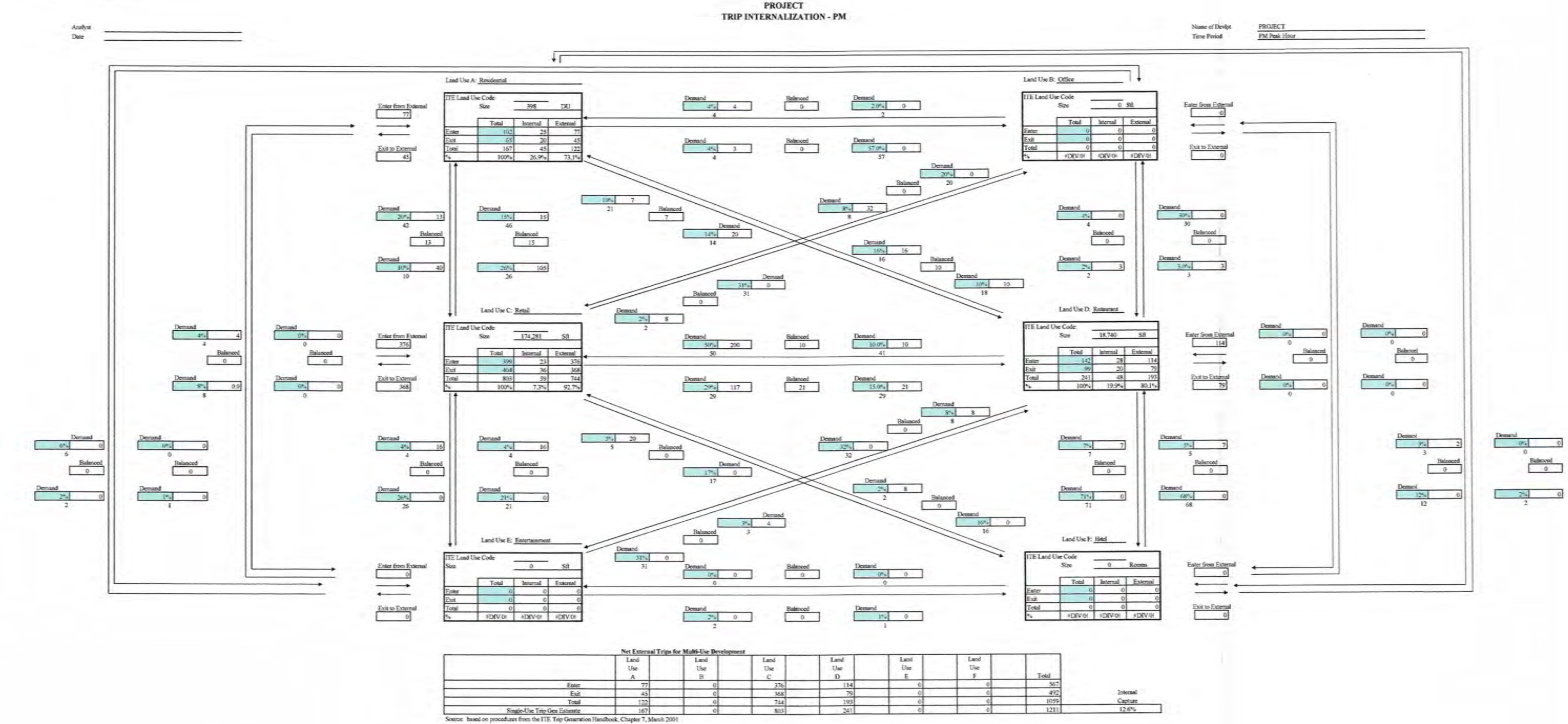


Table 13 – Road Improvement Needs by Land Use Type

Land Use	Unit	Trips	Trip				New Rds (Ln Ft)
			Length	% New	Adj %	VMT	
Residential:							
800 FT² & Under	Dwelling	5.34	5.63	100%	35%	10.66	6.13
801 to 1,100	Dwelling	5.40	5.63	100%	35%	10.78	6.19
1,101 to 2,300	Dwelling	6.63	5.63	100%	35%	13.23	7.60
2,301 & Over	Dwelling	9.57	5.63	100%	35%	19.10	10.98
Non-Residential:							
Hotel/Motel	Room	8.17	5.57	90%	35%	14.52	8.34
RV Park	Per Lot	4.00	5.57	90%	35%	7.11	4.09
Nursing Home	1000 s.f.	7.58	3.46	100%	35%	9.30	5.34
ACLF	1000 s.f.	2.66	3.46	100%	35%	3.26	1.88
Medical Office	1000 s.f.	36.13	3.46	90%	35%	39.88	22.92
Bank Walk In	1000 s.f.	69.60	3.46	53%	35%	45.25	26.00
Bank w/Drive In	1000 s.f.	148.15	3.46	53%	35%	96.31	55.35
Office Under 100,000 FT²	1000 s.f.	13.34	5.63	90%	35%	23.96	13.77
Office 100,000 to 199,999 FT²	1000 s.f.	11.38	5.63	90%	35%	20.44	11.75
Office 200,000 to 399,999 FT²	1000 s.f.	11.44	5.63	90%	35%	20.55	11.81
Office 400,000 TO 599,999 FT²	1000 s.f.	11.10	5.63	90%	35%	19.94	11.46
Office 600,000 TO 799,999 FT²	1000 s.f.	10.93	5.63	90%	35%	19.63	11.28
Office 800,000 TO 999,999 FT²	1000 s.f.	10.83	5.63	90%	35%	19.45	11.18
Office 1,000,000 Ft² or Larger	1000 s.f.	10.75	5.63	90%	35%	19.31	11.10
Manufacturing	1000 s.f.	3.82	5.63	90%	35%	6.86	3.94
Warehouse	1000 s.f.	3.56	5.63	90%	35%	6.40	3.68
Mini-Warehouse	1000 s.f.	2.50	5.63	90%	35%	4.49	2.58
Gen. Industrial	1000 s.f.	6.97	5.63	90%	35%	12.52	7.20
Retail Under 50,000 FT²	1000 s.f.	84.76	1.50	52%	35%	23.44	13.47
Retail 50,000 to 99,999 FT²	1000 s.f.	67.92	2.50	52%	35%	31.30	17.99
Retail 100,000 to 199,999 FT²	1000 s.f.	53.28	3.00	61%	35%	34.57	19.87
Retail 200,000 to 399,999 FT²	1000 s.f.	41.80	4.00	74%	35%	43.86	25.21
Retail 400,000 TO 599,999 FT²	1000 s.f.	36.27	5.73	77%	35%	56.73	32.60
Retail 600,000 TO 799,999 FT²	1000 s.f.	32.80	6.87	79%	35%	63.11	36.27
Retail 800,000 TO 999,999 FT²	1000 s.f.	30.33	8.00	80%	35%	68.81	39.55
Retail 1,000,000 Ft² or Larger	1000 s.f.	28.06	8.00	81%	35%	64.46	37.05
Gasoline/Service Station	Fuel Pstrn	168.56	1.50	50%	35%	44.82	25.76
Auto Sales & Repair	1000 s.f.	33.34	3.46	85%	35%	34.76	19.98
Restaurant	1000 s.f.	89.95	3.29	56%	35%	58.75	33.76
Fast Food Restaurant	1000 s.f.	496.12	1.50	51%	35%	134.54	77.33
Car Wash	1000 s.f.	108.00	3.46	100%	35%	132.47	76.13
Convenience Store w/o Gas	1000 s.f.	737.99	1.50	39%	35%	153.05	87.96
Convenience Store w/Gas	1000 s.f.	845.60	1.50	39%	35%	175.36	100.79
Pharmacy w/Drive Thru	1000 s.f.	88.16	3.46	50%	35%	54.07	31.07
Golf Course	Hole	35.74	3.21	95%	35%	38.64	22.21
Racquet Club	1000 s.f.	14.03	3.21	95%	35%	15.17	8.72
Parks	Acre	2.28	3.21	95%	35%	2.47	1.42
Tennis Court	Court	31.04	3.21	95%	35%	33.56	19.29
Marina	Slip	2.96	3.37	95%	35%	3.36	1.93
Boat Storage	Slip	2.96	3.37	95%	35%	3.36	1.93
Post Office	1000 s.f.	108.19	3.21	90%	35%	110.80	63.68
Library	1000 s.f.	56.24	5.63	90%	35%	101.02	58.06
Day Care Center	1000 s.f.	79.26	3.46	50%	35%	48.61	27.94
Hospital	1000 s.f.	16.50	5.63	90%	35%	29.64	17.03
House of Worship	1000 s.f.	9.11	5.36	95%	35%	16.45	9.45
Movie Theatre	1000 s.f.	78.06	3.21	95%	35%	84.39	48.50
Elem School	1000 s.f.	15.43	5.36	100%	35%	29.32	16.85
Middle School	1000 s.f.	13.78	5.36	100%	35%	26.18	15.05
High School	1000 s.f.	12.89	5.36	100%	35%	24.49	14.08
Fitness Center	1000 s.f.	32.93	3.46	95%	35%	38.37	22.05

Source: Institute of Transportation Engineers Trip Generation - 8th Edition
Palm Beach County Trip Generation Rates - September 1, 2011
Walter H. Keller, Inc.

Allowable Pass-By Calculation

Segment	From	To	Existing 2019 AADT	Existing 2019 2-Way Peak Hour Trips	Growth Rate	2024 Daily Trips	Daily Pass-By Allowed	2024 2-Way Peak Hour Trips	Peak Hour Pass-By Allowed
Kanner Hwy	Indian St	Monterey Rd	21116	1631	1.005	21649	2164.9	1672	167.2
Willoughby Blvd	Indian St	Monterey Rd	11100	1280	1.037	13311	1331.1	1535	153.5
Indian St	Kanner Hwy	Willoughby Blvd	29306	2771	1.047	36871	3687.1	3486	348.6
Total:							7183		669

APPENDIX D

MARTIN COUNTY 2019 ROADWAY LEVEL OF SERVICE INVENTORY REPORT

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	Class I: 4-Ln Divided	2000	23,957	1,622	C	6.1%
CR-723 (Savanna Rd)	CR-707	NE 24th St	Class I: 2-Ln Undivided	880	9,135	384	C	0.5%
CR-723 (Savanna Rd)	NE 24th St	CR-707A (Jensen Beach Blvd.)	Class I: 2-Ln Undivided	880	9,631	471	C	1.0%
CR-726 (Citrus Blvd)	SR-710	Greenridge Ln	Uninterrupted Rural Hwy: 2-Ln Undivided	740	2,783	126	A/B	0.5%
CR-726 (Citrus Blvd)	Greenridge Ln	CR-76A	Uninterrupted Rural Hwy: 2-Ln Undivided	740	2,488	137	A/B	0.5%
CR-76A (Citrus Blvd.)	CR-726	SR-714	Transitional 2-Ln Uninter /Undivided Flow	1200	4,269	283	A/B	8.0%
CR-76A (SW 96th St)	CR-726	Pennsylvania Ave	Transitional 2-Ln Uninter /Undivided Flow	1200	3,845	195	A/B	8.0%
CR-76A (SW 96th St)	Pennsylvania Ave	SR-76	Class I: Transitional 2-Ln Undivided	800	8,086	492	C	2.5%
CR-A1A (Dixie Hwy)	SR-5	CR-708	Class II: 2-Ln Undivided	750	3,416	192	C	4.7%
CR-A1A (Dixie Hwy)	CR-708	Osprey St	Class I: 2-Ln Undivided	880	7,552	477	C	3.7%
CR-A1A (Dixie Hwy)	Osprey St	Heritage Blvd	2-Ln Uninter /Undivided Flow	1190	6,595	393	A/B	3.1%
CR-A1A (Dixie Hwy)	Heritage Blvd	Cove Rd	2-Ln Uninter /Undivided Flow	1190	7,147	346	A/B	2.9%
CR-A1A (Dixie Hwy)	Cove Rd	Salerno Rd	Class II: 2-Ln Divided	790	12,131	617	D	3.1%
CR-A1A (Dixie Hwy)	Salerno Rd	St. Lucie Blvd	Class II: 2-Ln Undivided	750	17,261	835	F	3.3%
CR-A1A (Dixie Hwy)	St Lucie Blvd	Jefferson St	Class II: 2-Ln Undivided	750	14,108	748	D	2.9%
CR-A1A (Dixie Hwy)	Jefferson St	Indian St	Class II: 4-Ln Divided	1630	18,389	848	D	2.2%
CR-A1A (Dixie Hwy)	Indian St	SR-714	Class I: 4-Ln Divided	2000	17,161	824	C	3.1%
CR-A1A (Dixie Hwy)	SR-714	SE Fifth St	2-Ln Undivided Non-State	675	6,710	346	D	1.6%
Farm Rd	Dr Martin Luther King Jr Dr	Palm Wy	Class II: 2-Ln Undivided	750	2,817	124	C	3.2%
Fox Brown Rd	SR-710	CR-714	Uninterrupted Rural Hwy: 2-Ln Undivided	740	347	16	A/B	0.0%
Goldenrod Rd	Britt Rd	SR-732	Class II: 4-Ln Divided	1630	4,810	278	A/B	0.5%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
Goldenrod Rd	SR-732	SR-5	Class II: 2-Ln Undivided	750	6,877	402	D	1.9%
Goldenrod Rd	SR-5	Westmoreland Blvd	Class II: 2-Ln Undivided	750	4,635	295	C	3.0%
Gomez Ave	CR-708	Crossrip St	Class II: 2-Ln Undivided	750	3,732	199	C	1.4%
Gomez Ave	Crossrip St	Osprey St	Class II: 2-Ln Undivided	750	1,065	61	C	0.5%
Green River Parkway	Dixie Hwy	Baker Rd	Class II: 2-Ln Undivided	750	6,833	351	C	3.1%
Green River Parkway	Baker Rd	SR-732	Class I: 2-Ln Undivided	880	8,341	596	C	4.3%
Green River Parkway	SR-732	St. Lucie County	2-Ln Uninter /Undivided Flow	1190	8,667	576	C	1.8%
Horseshoe Point Rd	CR-A1A	Kubin Ave	2-Ln Undivided Non-State	675	5,647	285	C	0.5%
Indian St	SR-76	Willoughby Blvd	Class I: 4-Ln Divided	2000	29,306	1,729	C	4.7%
Indian St	Willoughby Blvd	SR-5	Class I: 4-Ln Divided	2000	27,852	1,236	C	2.8%
Indian St	SR-5	Commerce Ave	Class I: 4-Ln Divided	2000	22,368	995	C	0.5%
Indian St	Commerce Ave	CR-A1A	Class I: 4-Ln Divided	2000	25,515	1,199	C	2.5%
Indian St	CR-A1A	St Lucie Blvd	2-Ln Undivided Non-State	675	7,051	346	D	1.1%
Indian River Dr	Palmer St	CR-707	Class II: 2-Ln Undivided	750	7,509	374	D	3.7%
Island Way	Palm Beach County	Jupiter Road	Transitional 2-Ln Uninter /Undivided Flow	1200	4,493	302	A/B	3.4%
Island Way	Jupiter Road	Country Club Dr	Class II: 2-Ln Undivided	750	4,855	213	C	0.5%
Jack James Rd	SR-76	Blue Water Wy	Class II: 2-Ln Undivided	750	3,132	294	C	0.5%
Lares St	CR-708	CR-A1A	2-Ln Undivided Non-State	675	3,349	235	C	0.6%
Little Club Wy	Country Club Dr	Wooden Bridge Wy	2-Ln Undivided Non-State	675	2,273	103	C	0.5%
Locks Rd	Canal St	SR-76	2-Ln Undivided Non-State	675	3,667	232	C	2.0%
MacArthur Blvd	Sailfish Point	SR-A1A	2-Ln Undivided Non-State	675	5,989	368	D	6.8%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
SR-5 (US-1)	Britt Rd	SR-732	Class I: 7-Ln Divided	3530	55,878	2,898	C	0.8%
SR-5 (US-1)	SR-732	Westmoreland Blvd	Class I: 8-Ln Divided	4040	64,147	3,522	C	0.5%
SR-5 (US-1)	Westmoreland Blvd	St Lucie County	Class I: 8-Ln Divided	4040	60,343	2,721	C	0.5%
SR-710 (Warfield Blvd)	Okeechobee County	Fox Brown Rd	Uninterrupted Rural Hwy: 2-Ln Undivided	670	6,348	257	C	4.8%
SR-710 (Warfield Blvd)	Fox Brown Rd	CR-609 (Allapattah)	2-Ln Uninter /Undivided Flow	840	7,419	310	A/B	3.0%
SR-710 (Warfield Blvd)	CR-609 (Allapattah)	Van Buren	Class I: 2-Ln Undivided	870	12,335	694	C	4.0%
SR-710 (Warfield Blvd)	Van Buren	CR-726 (Citrus)	Class II: 4-Ln Divided	1710	12,335	694	A/B	4.0%
SR-710 (Warfield Blvd)	CR-726	SR-76 (Kanner)	Transitional 4-Ln Uninter /Divided Flow	2450	11,001	431	A/B	3.4%
SR-710 (Warfield Blvd)	SR-76	Palm Beach County	Transitional 4-Ln Uninter /Divided Flow	2450	6,959	258	A/B	1.4%
SR-714 (Martin Hwy)	I-95	CR-76A (Citrus)	Transitional 2-Ln Uninter /Undivided Flow	1200	13,786	735	C	5.1%
SR-714 (Martin Hwy)	CR-76A (Citrus)	Florida's Turnpike	Class I: 4-Ln Divided	2000	21,291	1,136	C	4.9%
SR-714 (Martin Downs Blvd)	Florida's Turnpike	CR-713	Class I: 4-Ln Divided	2000	20,695	1,038	C	1.7%
SR-714 (Martin Downs Blvd)	CR-713	Matheson Ave	Class I: 4-Ln Divided	2000	27,291	1,267	C	0.5%
SR-714 (Martin Downs Blvd)	Matheson Ave	Mapp Rd	Class I: 4-Ln Divided	2000	32,789	1,738	C	0.5%
SR-714 (Palm City Bridge)	Mapp Rd	SR-76	Class I: 4-Ln Divided	2000	34,751	1,989	D	0.5%
SR-714 (Monterey Rd)	SR-76	Willoughby Blvd	Class I: 4-Ln Divided	2000	22,465	1,274	C	0.5%
SR-714 (Monterey Rd)	Willoughby Blvd	Monterey Extension	Class I: 4-Ln Divided	2000	24,716	1,186	C	0.6%
SR-714 (Monterey Rd)	Monterey Extension	SR-5	Class I: 4-Ln Divided	2000	18,136	926	C	0.8%
SR-714 (Monterey Rd)	SR-5	CR-A1A	Class II: 4-Ln Divided	1630	21,333	1,099	D	0.5%
SR-714 (Monterey Rd)	CR-A1A	SR-A1A	Class I: 4-Ln Divided	1910	19,430	1,031	C	1.1%
SR-732 (Causeway Blvd)	CR-707	SR-A1A	2-Ln Uninter /Undivided Flow	1190	12,645	786	C	0.5%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
SR-732 (Jensen Beach Blvd)	SR-5	Green River Pkwy	Class I: 4-Ln Divided	2000	25,949	1,349	C	0.5%
SR-732 (Jensen Beach Blvd)	Green River Pkwy	CR-723	Class I: 4-Ln Divided	2000	23,804	1,154	C	0.5%
SR-76 (Kanner Hwy)	SR-15	SR-710	Uninterrupted Rural Hwy: 2-Ln Undivided	740	2,134	111	A/B	6.1%
SR-76 (Kanner Hwy)	SR-710	CR-708	Uninterrupted Rural Hwy: 2-Ln Undivided	740	3,412	151	A/B	0.9%
SR-76 (Kanner Hwy)	CR-708	CR-711/CR-76A	Transitional 2-Ln Uninter /Undivided Flow	1200	2,889	168	A/B	0.5%
SR-76 (Kanner Hwy)	CR-711/CR76A	Locks Rd	Class I: 4-Ln Divided	2000	13,401	831	C	1.7%
SR-76 (Kanner Hwy)	Locks Rd	Jack James	Class I: 4-Ln Divided	2000	19,231	920	C	0.5%
SR-76 (Kanner Hwy)	Jack James	Cove Rd	Class I: 6-Ln Divided	3020	43,880	2,372	C	0.6%
SR-76 (Kanner Hwy)	Cove Rd	Salerno Rd	Class I: 6-Ln Divided	3020	34,493	1,495	C	1.0%
SR-76 (Kanner Hwy)	Salerno Rd	Indian St	Class I: 6-Ln Divided	3020	30,912	1,293	C	2.6%
SR-76 (Kanner Hwy)	Indian St	SR-714	Class I: 6-Ln Divided	3020	21,116	993	C	0.5%
SR-76 (Kanner Hwy)	SR-714	SR-5	Class I: 6-Ln Divided	3020	25,158	1,006	C	0.5%
SR-A1A (Ocean Blvd)	SR-714	St Lucie Blvd	Class II: 4-Ln Divided	1630	19,437	1,081	D	0.6%
SR-A1A (Ocean Blvd)	St Lucie Blvd	Sewalls Point Rd	Class I: 4-Ln Divided	2000	23,060	1,127	C	1.0%
SR-A1A (Ocean Blvd)	Sewalls Point Rd	Macarthur Blvd	Class I: 2-Ln Divided	925	13,597	845	C	0.5%
SR-A1A (Ocean Blvd)	MacArthur Blvd	SR-732	2-Ln Uninter /Undivided Flow	1190	8,532	407	A/B	3.1%
SR-A1A (Ocean Blvd)	SR-732	St Lucie County	2-Ln Uninter /Undivided Flow	1190	14,752	870	D	1.8%
St Lucie Blvd	CR-A1A	Indian St	2-Ln Undivided Non-State	675	3,305	172	C	0.5%
St Lucie Blvd	Indian St	SR-A1A	2-Ln Undivided Non-State	675	7,524	429	D	3.3%
Westmoreland Blvd	St Lucie County	SR-5	Class II: 2-Ln Divided	790	12,271	601	D	1.0%
Willoughby Blvd	Cove Rd	Salerno Rd	Class I: 2-Ln Undivided	880	4,332	214	C	8.0%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
Willoughby Blvd	Salerno Rd	Pomeroy St	Class I: 2-Ln Undivided	880	9,007	544	C	5.4%
Willoughby Blvd	Pomeroy St	Indian St	Class I: 4-Ln Divided	2000	11,709	597	C	8.0%
Willoughby Blvd	Indian St	SR-714	Class I: 4-Ln Divided	2000	11,100	666	C	3.7%
Wright Blvd	SR-5	Dixie Highway	Class II: 2-Ln Undivided	750	10,460	555	D	4.3%

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FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2019 HISTORICAL AADT REPORT

COUNTY: 89 - MARTIN

SITE: 0114 - SR 76/COLORADO AVE - SW OF MONTEREY RD (COUNTY LINK: 132)

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2019	26000 C	N 13500	S 12500	9.00	62.40	5.50
2018	27500 C	N 13500	S 14000	9.00	61.40	5.50
2017	25500 C	N 13000	S 12500	9.00	61.80	5.50
2016	26000 C	N 13000	S 13000	9.00	62.10	4.90
2015	26000 C	N 13000	S 13000	9.00	62.30	4.90
2014	23000 C	N 12000	S 11000	9.00	62.60	5.30
2013	26500 C	N 13500	S 13000	9.00	62.40	5.90
2012	25000 C	N 12500	S 12500	9.00	60.30	5.90
2011	27500 C	N 14000	S 13500	9.00	59.50	3.40
2010	27500 C	N 14000	S 13500	9.38	58.28	3.40
2009	28000 C	N 15000	S 13000	9.15	62.05	3.40
2007	28000 C	N 14000	S 14000	9.15	59.55	4.90
2006	27000 C	N 14000	S 13000	9.10	57.64	8.50
2005	26000 C	N 13500	S 12500	9.00	59.00	6.10
2004	28000 C	N 14500	S 13500	8.90	61.20	6.10

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2019 HISTORICAL AADT REPORT

COUNTY: 89 - MARTIN

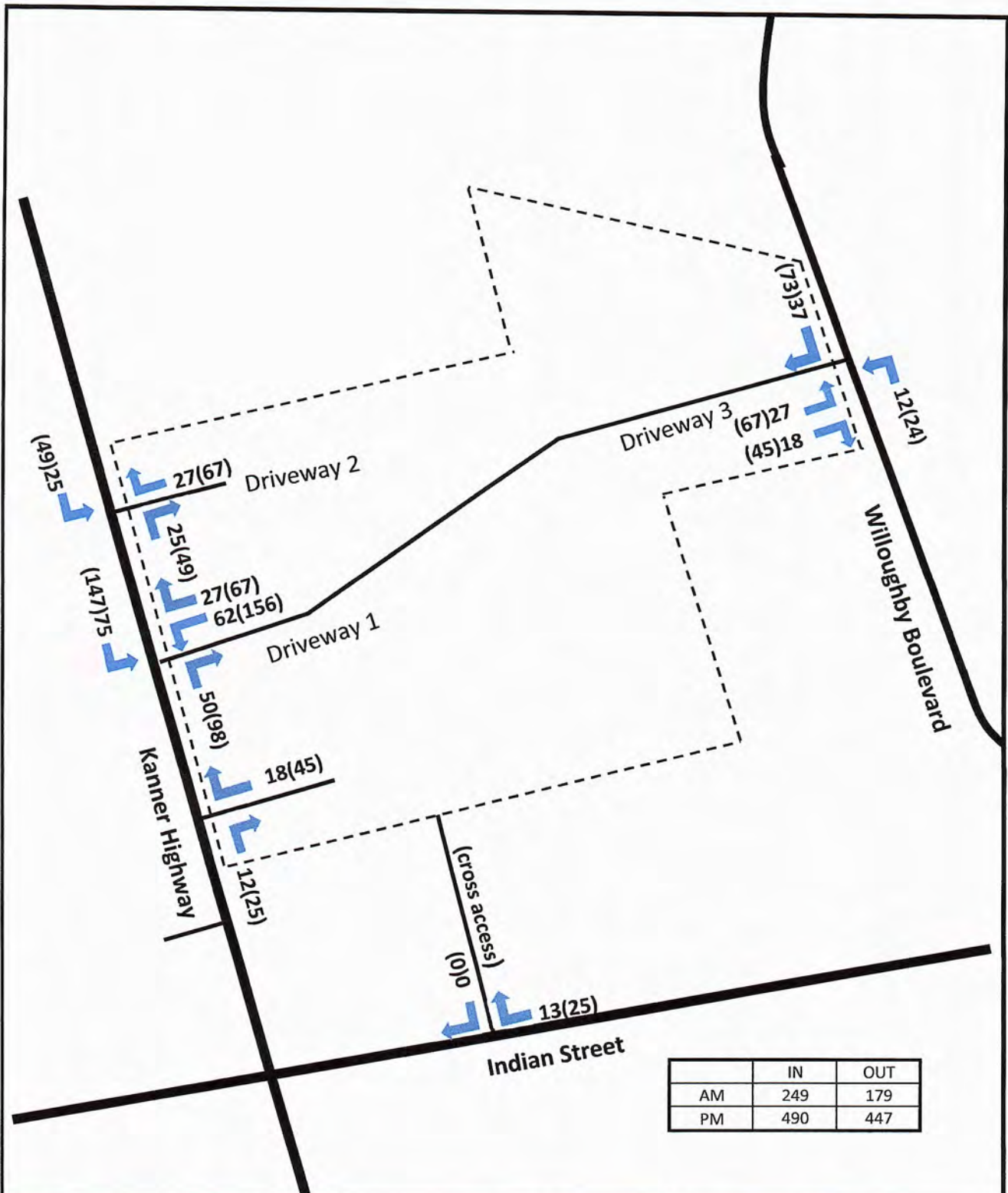
SITE: 0063 - SE WILLOUGHBY BLVD SOUTN FO INDIAN ST(COUNTY LINK: 160)

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2019	14900 F	N 7400	S 7500	9.00	62.40	13.80
2018	14700 C	N 7300	S 7400	9.00	61.40	6.10
2017	9900 V	N 4900	S 5000	9.00	61.80	3.90
2016	9700 R	N 4800	S 4900	9.00	62.10	3.80
2015	9300 T	N 4600	S 4700	9.00	62.30	6.50
2014	9100 S	N 4500	S 4600	9.00	62.60	10.10
2013	8900 F	N 4400	S 4500	9.00	62.40	11.00
2012	8700 C	N 4300	S 4400	9.00	60.30	10.50

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

APPENDIX E

DRIVEWAY DATA AND ANALYSES



	IN	OUT
AM	249	179
PM	490	447



NTS
22 SE Seminole St
Stuart, FL, 34994

Job #: MR18031.0

Date: 6/25/20

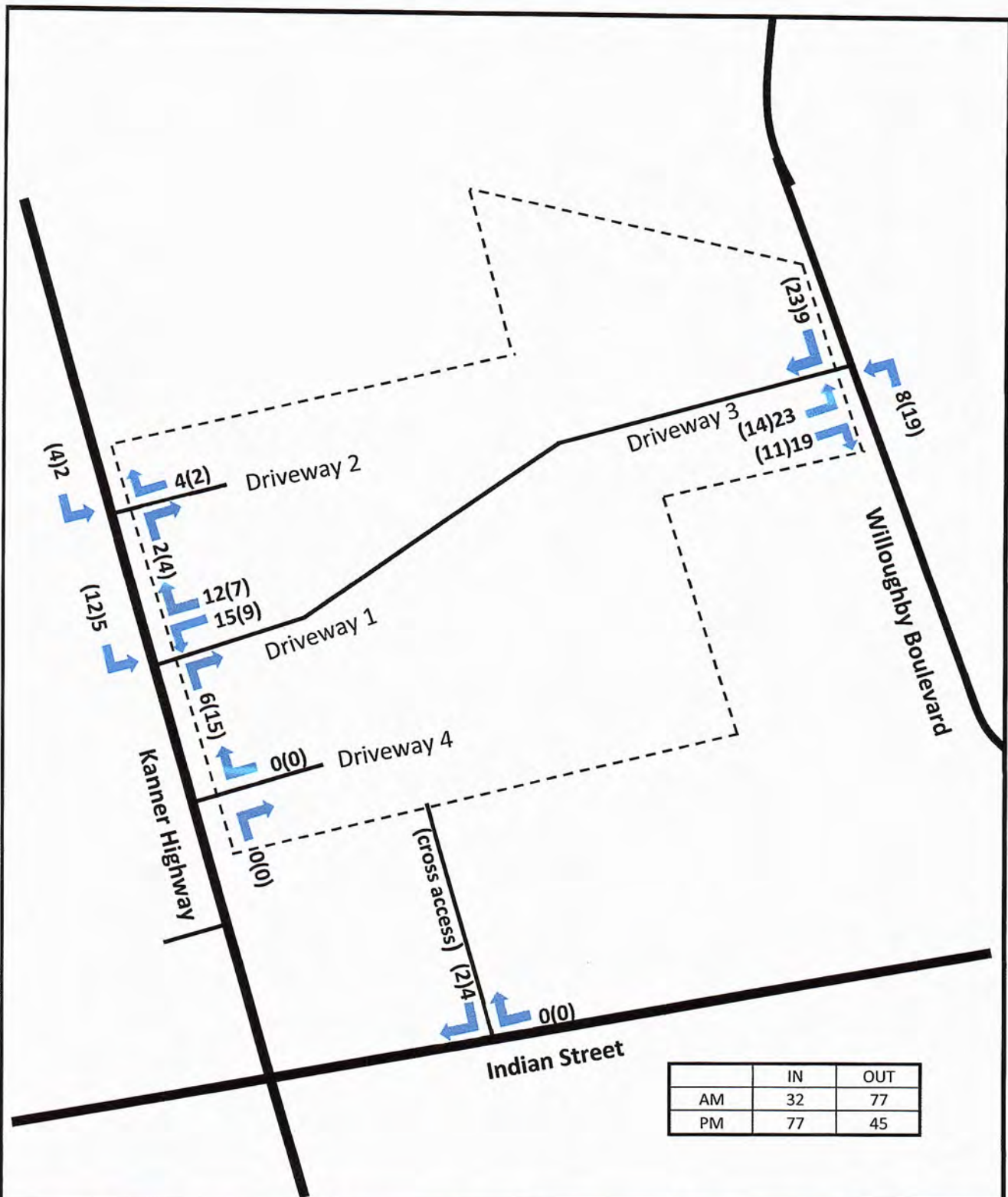
Legend

XX(XX) = AM(PM)
 = Project Location
 = Direction

Appendix E

Retail Driveway Volumes

M & M Retail



  NTS 22 SE Seminole St Stuart, FL, 34994 Job #: MR18031.0 Date: 4/21/21	Legend XX(X) = AM(PM) --- = Project Location ← = Direction	Appendix E Residential Driveway Volumes M & M Retail
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COUNTY: 89
STATION: 0114
DESCRIPTION: SR 76/COLORADO AVE - SW OF MONTEREY RD(COUNTY LINK
START DATE: 03/20/2019
START TIME: 0000

TIME	DIRECTION: N				TOTAL	DIRECTION: S				TOTAL	COMBINED TOTAL	
	1ST	2ND	3RD	4TH		1ST	2ND	3RD	4TH			
0000	36	36	28	16	116	19	12	20	19	70	186	
0100	28	20	19	20	87	7	11	7	11	36	123	
0200	14	11	4	7	36	19	16	4	11	50	86	
0300	6	10	17	16	49	14	19	19	31	83	132	
0400	17	20	25	25	87	30	37	43	50	160	247	
0500	23	38	37	74	172	74	98	85	139	396	568	
0600	72	95	148	134	449	150	172	175	206	703	1152	
0700	163	230	254	298	945	277	296	253	269	1095	2040	
0800	284	329	256	218	1087	265	186	240	198	889	1976	
0900	230	218	224	229	901	172	188	184	212	756	1657	
1000	245	224	214	224	907	215	197	224	191	827	1734	
1100	241	234	229	284	988	210	203	229	218	860	1848	
1200	245	270	259	275	1049	212	203	194	188	797	1846	
1300	209	202	238	217	866	203	234	198	179	814	1680	
1400	256	287	228	241	1012	187	248	218	202	855	1867	
1500	270	250	254	276	1050	208	239	206	222	875	1925	
1600	317	281	287	240	1125	226	203	212	244	885	2010	
1700	275	260	271	292	1098	251	215	222	215	903	2001	
1800	260	234	208	184	886	168	157	160	145	630	1516	
1900	198	175	160	124	657	176	142	120	125	563	1220	
2000	156	134	110	88	488	106	115	131	94	446	934	
2100	79	85	82	70	316	108	101	85	77	371	687	
2200	72	68	54	54	248	62	52	41	46	201	449	
2300	22	52	59	52	185	32	37	23	20	112	297	
24-HOUR TOTALS:					14804						13377	28181

TURNING MOVEMENT VOLUME COUNTS

N/S STREET: Kanner Hwy
 E/W STREET: Driveway 1
 FILENAME: 3/20/2019
 COUNT DATE: 12/16/2020
 REPORT DATE: 12/16/2020
 DAY: Wednesday
 ANALYSIS YEAR: 2024
 CITY: Stuart
 INTERSECTION:
 CONTROL:

15 Min Period	Northbound				Southbound				Eastbound				Westbound			
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL	ONE HOUR SUM		
7:00-7:15	0	163	0	0	277	0	0	0	0	0	0	0	0	440	2010	
7:15-7:30	0	230	0	0	296	0	0	0	0	0	0	0	0	526	2149	
7:30-7:45	0	254	0	0	253	0	0	0	0	0	0	0	0	507	2118	
7:45-8:00	0	298	0	0	269	0	0	0	0	0	0	0	0	567	2127	
8:00-8:15	0	284	0	0	205	0	0	0	0	0	0	0	0	549	1976	
8:15-8:30	0	329	0	0	185	0	0	0	0	0	0	0	0	515		
8:30-8:45	0	256	0	0	240	0	0	0	0	0	0	0	0	496		
8:45-9:00	0	218	0	0	198	0	0	0	0	0	0	0	0	416		

AM PEAK HOUR IS FROM: 7:15AM TO 8:15AM
 Volumes: 1066 0 0 1083 0 0 0 0 0 0 0 0 0 0 2149
 Season Factor: 1 1083 0 0 1083 0 0 0 0 0 0 0 0 0 2149
 Growth: 0 1099 0 0 1110 0 0 0 0 0 0 0 0 0 2203
 In/Out: - IN/OUT - - - - - - - - - - - - - - - -
 Res Percentage: 0% 20% 15% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%
 Retail Percentage: 0% 5%/10% 20% 30% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%
 PROJECT: 0 30 56 80 0 0 0 0 0 0 0 0 0 0 0
 MMHS: 0 30 56 80 0 0 0 0 0 0 0 0 0 0 0
 Kanner ALF: 51

Total	0	1123	56	80	1110	0	0	0	0	0	136	0	38	2543		
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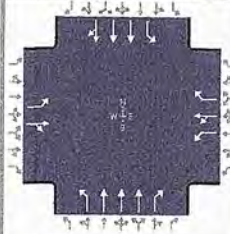
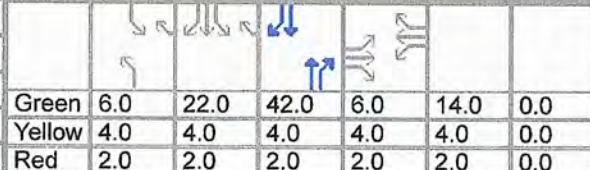
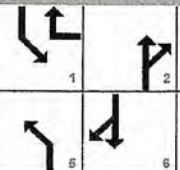
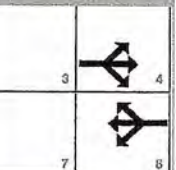
15 Min Period	Northbound				Southbound				Eastbound				Westbound			
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL	ONE HOUR SUM		
4:00-4:15	0	317	0	0	226	0	0	0	0	0	0	0	0	543	2010	
4:15-4:30	0	281	0	0	203	0	0	0	0	0	0	0	0	484	1993	
4:30-4:45	0	287	0	0	212	0	0	0	0	0	0	0	0	499	1984	
4:45-5:00	0	240	0	0	244	0	0	0	0	0	0	0	0	484	1978	
5:00-5:15	0	275	0	0	251	0	0	0	0	0	0	0	0	526	2001	
5:15-5:30	0	260	0	0	215	0	0	0	0	0	0	0	0	475		
5:30-5:45	0	271	0	0	222	0	0	0	0	0	0	0	0	493		
5:45-6:00	0	292	0	0	215	0	0	0	0	0	0	0	0	507		

PM PEAK HOUR IS FROM: 4:00PM TO 5:00PM
 Volumes: 1125 0 0 885 0 0 0 0 0 0 0 0 0 0 2010
 Season Factor: 0 1125 0 0 885 0 0 0 0 0 0 0 0 0 2010
 Growth: 0 1153 0 0 907 0 0 0 0 0 0 0 0 0 2051
 In/Out: - IN/OUT - - - - - - - - - - - - - - - -
 Res Percentage: 0% 20% 15% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%
 Retail Percentage: 0% 5%/10% 20% 30% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%
 PROJECT: 0 69 113 159 0 0 0 0 0 0 0 0 0 0 0
 MMHS: 0 69 113 159 0 0 0 0 0 0 0 0 0 0 0
 Kanner ALF: 48

Total	0	1223	113	159	907	0	0	0	0	0	249	0	74	2725		
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Lane call out subjected to FOOT permitting.

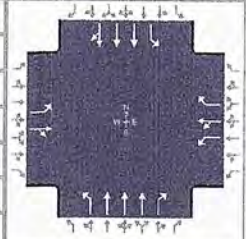
HCS7 Signalized Intersection Results Summary

General Information						Intersection Information									
Agency	O'Rourke Engineering & Planning					Duration, h	0.250								
Analyst	James Kemp		Analysis Date	Apr 21, 2021		Area Type	Other								
Jurisdiction	Martin County		Time Period	AM		PHF	0.95								
Urban Street	Kanner Highway		Analysis Year	2024		Analysis Period	1> 7:00								
Intersection	Kanner & Driveway 1		File Name	Kanner and Driveway 1 - AM - Costco - 4.21.21.xus											
Project Description	Costco														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				2	2	2	136	1	38	10	1123	56	80	1110	1
Signal Information															
Cycle, s	120.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On												
Green	6.0	22.0	42.0	6.0	14.0	0.0									
Yellow	4.0	4.0	4.0	4.0	4.0	0.0									
Red	2.0	2.0	2.0	2.0	2.0	0.0									
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4		8	5	2	1	6				
Case Number					10.0		9.0	2.0	3.0	2.0	4.0				
Phase Duration, s					12.0		20.0	12.0	48.0	40.0	76.0				
Change Period, (Y+R c), s					6.0		6.0	6.0	6.0	6.0	6.0				
Max Allow Headway (MAH), s					3.1		3.0	3.0	0.0	3.0	0.0				
Queue Clearance Time (g s), s					2.3		6.4	2.7		6.2					
Green Extension Time (g e), s					0.0		0.2	0.0	0.0	0.1	0.0				
Phase Call Probability					1.00		1.00	1.00		1.00					
Max Out Probability					0.17		0.01	0.45		0.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h				2	4		72	73	40	11	1182	59	84	780	390
Adjusted Saturation Flow Rate (s), veh/h/ln				1810	1743		1810	1811	1610	1810	1725	1610	1810	1900	1899
Queue Service Time (g s), s				0.1	0.3		4.4	4.4	1.8	0.7	23.1	3.0	4.2	12.9	12.9
Cycle Queue Clearance Time (g c), s				0.1	0.3		4.4	4.4	1.8	0.7	23.1	3.0	4.2	12.9	12.9
Green Ratio (g/C)				0.05	0.05		0.12	0.12	0.40	0.05	0.35	0.35	0.28	0.58	0.58
Capacity (c), veh/h				90	87		211	211	644	90	1811	564	513	2217	1108
Volume-to-Capacity Ratio (X)				0.023	0.048		0.339	0.344	0.062	0.116	0.653	0.105	0.164	0.352	0.352
Back of Queue (Q), ft/ln (95 th percentile)				2.7	5.4		87.6	88.9	30.5	13.6	365	52.4	81.1	223.3	227.8
Back of Queue (Q), veh/ln (95 th percentile)				0.1	0.2		3.5	3.6	1.2	0.5	14.6	2.1	3.2	8.9	9.1
Queue Storage Ratio (RQ) (95 th percentile)				0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d 1), s/veh				54.2	54.3		48.7	48.8	22.2	54.5	32.9	26.3	32.3	13.1	13.1
Incremental Delay (d 2), s/veh				0.0	0.1		0.4	0.4	0.0	0.2	1.8	0.4	0.1	0.4	0.9
Initial Queue Delay (d 3), s/veh				0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				54.3	54.4		49.1	49.1	22.2	54.7	34.7	26.7	32.4	13.5	14.0
Level of Service (LOS)				D	D		D	D	C	D	C	C	C	B	B
Approach Delay, s/veh / LOS				54.3		D	43.3		D	34.5		C	14.9		B
Intersection Delay, s/veh / LOS				26.0						C					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.74		C	2.62		C	2.12		B	1.89		B
Bicycle LOS Score / LOS				0.50		A	0.79		A	1.18		A	1.18		A

HCS7 Signalized Intersection Results Summary

General Information

Agency	O'Rourke Engineering & Planning			Intersection Information	
Analyst	James Kemp	Analysis Date	Mar 16, 2021	Duration, h	0.250
Jurisdiction	Martin County	Time Period	PM	Area Type	Other
Urban Street	Kanner Highway	Analysis Year	2024	PHF	0.95
Intersection	Kanner & Driveway 1	Analysis Period	1> 7:00	File Name	Kanner and Driveway 1 - PM - Costco - 4.21.21.xus
Project Description	Costco				



Demand Information

Approach Movement	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	2	2	2	249	1	74	10	1223	113	159	907	1

Signal Information

Cycle, s	120.0	Reference Phase	2
Offset, s	0	Reference Point	End
Uncoordinated	No	Simult. Gap E/W	On
Force Mode	Fixed	Simult. Gap N/S	On

Timer Results

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8	5	2	1	6
Case Number		10.0		9.0	2.0	3.0	2.0	4.0
Phase Duration, s		10.0		18.0	10.0	36.0	56.0	82.0
Change Period, (Y+R _c), s		6.0		6.0	6.0	6.0	6.0	6.0
Max Allow Headway (MAH), s		3.1		3.0	3.0	0.0	3.0	0.0
Queue Clearance Time (g _s), s		2.3		10.5	2.7		7.6	
Green Extension Time (g _e), s		0.0		0.1	0.0	0.0	0.3	0.0
Phase Call Probability		1.00		1.00	1.00		1.00	
Max Out Probability		1.00		1.00	1.00		0.00	

Movement Group Results

Approach Movement	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate (v), veh/h	2	4		131	132	78	11	1287	66	167	637	318
Adjusted Saturation Flow Rate (s), veh/h/ln	1810	1743		1810	1810	1610	1810	1725	1610	1810	1900	1899
Queue Service Time (g _s), s	0.1	0.3		8.4	8.5	2.9	0.7	29.8	3.9	5.6	4.0	4.0
Cycle Queue Clearance Time (g _c), s	0.1	0.3		8.4	8.5	2.9	0.7	29.8	3.9	5.6	4.0	4.0
Green Ratio (g/C)	0.03	0.03		0.10	0.10	0.52	0.03	0.25	0.25	0.42	0.63	0.63
Capacity (c), veh/h	60	58		181	181	832	60	1294	403	754	2407	1203
Volume-to-Capacity Ratio (X)	0.035	0.072		0.724	0.730	0.094	0.175	0.995	0.165	0.222	0.265	0.265
Back of Queue (Q), ft/ln (95 th percentile)	2.8	5.6		193.3	195.2	46.3	14	505.5	71.1	98.5	60.5	64.9
Back of Queue (Q), veh/ln (95 th percentile)	0.1	0.2		7.7	7.8	1.9	0.6	20.2	2.8	3.9	2.4	2.6
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay (d ₁), s/veh	56.1	56.2		52.4	52.4	14.7	56.4	39.9	35.2	16.8	3.7	3.8
Incremental Delay (d ₂), s/veh	0.1	0.2		11.8	12.3	0.0	0.5	23.8	0.9	0.1	0.3	0.5
Initial Queue Delay (d ₃), s/veh	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	56.2	56.4		64.2	64.7	14.7	56.9	63.8	36.1	16.9	4.0	4.3
Level of Service (LOS)	E	E		E	E	B	E	E	D	B	A	A
Approach Delay, s/veh / LOS	56.3	E		53.1	D		62.4	E		6.0	A	
Intersection Delay, s/veh / LOS	38.9						D					

Multimodal Results

	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.81	C	2.62	C	2.13	B	1.88	B
Bicycle LOS Score / LOS	0.50	A	1.05	A	1.24	A	1.11	A

TURNING MOVEMENT VOLUME COUNTS

N/S STREET:	Kanner Hwy	CONTROL:	SW STREET, Driveway 2
FILENAME:	4/27/2017	CITY:	Stuart
COUNT DATE:	5/2/2017	INTERSECTION:	
REPORT DATE:	5/2/2017	ANALYSIS YEAR:	2024
DAY:	Thursday		

15 Min Period	Northbound			Southbound			Eastbound			Westbound			ONE HOUR SUM
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL
7:00-7:15	0	163	0	0	277	0	0	0	0	0	0	0	440
7:15-7:30	0	230	0	0	296	0	0	0	0	0	0	0	526
7:30-7:45	0	254	0	0	253	0	0	0	0	0	0	0	507
7:45-8:00	0	298	0	0	269	0	0	0	0	0	0	0	567
8:00-8:15	0	284	0	0	265	0	0	0	0	0	0	0	549
8:15-8:30	0	329	0	0	186	0	0	0	0	0	0	0	515
8:30-8:45	0	256	0	0	240	0	0	0	0	0	0	0	496
8:45-9:00	0	218	0	0	198	0	0	0	0	0	0	0	416

AM PEAK HOUR IS FROM: 7:15AM TO 8:15AM

Volumes	0	1066	0	0	1083	0	0	0	0	0	0	0	2149
Season Factor	0	1066	0	0	1083	0	0	0	0	0	0	0	2149
Growth	0	1093	0	0	1110	0	0	0	0	0	0	0	2203
In/Out	0	OUT	IN	IN	IN	0	0	0	0	0	0	0	OUT
Res Percentage	0%	15%	5%	5%	15%	0%	0%	0%	0%	0%	0%	0%	0%
Retail Percentage	0%	25%	10%	10%	30%	0%	0%	0%	0%	0%	0%	0%	0%
PROJECT	0	56	27	27	80	0	0	0	0	0	0	0	31

Seasonal Factor: 1
 Retail Residential
 Trips In: 249 32
 Trips Out: 179 77
 Growth Rate: 1.005
 Years Grown: 5

15 Min Period	Northbound			Southbound			Eastbound			Westbound			ONE HOUR SUM
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL
4:00-4:15	0	317	0	0	226	0	0	0	0	0	0	0	543
4:15-4:30	0	281	0	0	203	0	0	0	0	0	0	0	484
4:30-4:45	0	287	0	0	212	0	0	0	0	0	0	0	499
4:45-5:00	0	240	0	0	244	0	0	0	0	0	0	0	484
5:00-5:15	0	275	0	0	251	0	0	0	0	0	0	0	526
5:15-5:30	0	260	0	0	215	0	0	0	0	0	0	0	475
5:30-5:45	0	271	0	0	222	0	0	0	0	0	0	0	493
5:45-6:00	0	292	0	0	215	0	0	0	0	0	0	0	507

PM PEAK HOUR IS FROM: 4:00PM TO 5:00PM

Volumes	0	1125	0	0	885	0	0	0	0	0	0	0	2010
Season Factor	0	1125	0	0	885	0	0	0	0	0	0	0	2010
Growth	0	1153	0	0	907	0	0	0	0	0	0	0	2051
In/Out	0	OUT	IN	IN	IN	0	0	0	0	0	0	0	OUT
Res Percentage	0%	15%	5%	5%	15%	0%	0%	0%	0%	0%	0%	0%	0%
Retail Percentage	0%	25%	10%	10%	30%	0%	0%	0%	0%	0%	0%	0%	0%
PROJECT	0	119	53	53	159	0	0	0	0	0	0	0	69

Seasonal Factor: 1
 Retail Residential
 Trips In: 460 77
 Trips Out: 447 45
 Growth Rate: 1.005
 Years Grown: 5

Total 0 1272 53 53 1066 0 0 0 0 0 0 0 0 69 2513

HCS7 Two-Way Stop-Control Report

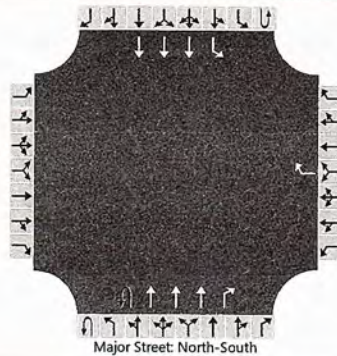
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	4/21/2021
Analysis Year	2024
Time Analyzed	AM
Intersection Orientation	North-South
Project Description	Costco

Site Information

Intersection	Kanner Hwy & Driveway 2
Jurisdiction	Martin County
East/West Street	Driveway 2
North/South Street	Kanner Hwy
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	0	1	1	0	3	1	0	1	3	0
Configuration								R	U		T	R		L	T	
Volume (veh/h)								31	0		1149	27	0	27	1190	
Percent Heavy Vehicles (%)								3	3				3	3		
Proportion Time Blocked								0.250						0.250		
Percent Grade (%)					0											
Right Turn Channelized					Yes				No							
Median Type Storage	Left Only								1							

Critical and Follow-up Headways

Base Critical Headway (sec)								7.1	5.6					5.3		
Critical Headway (sec)								7.16	5.66					5.36		
Base Follow-Up Headway (sec)								3.3	2.3					3.1		
Follow-Up Headway (sec)								3.33	2.33					3.13		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)								33	0					28		
Capacity, c (veh/h)								811	486					736		
v/c Ratio								0.04	0.00					0.04		
95% Queue Length, Q ₉₅ (veh)								0.1	0.0					0.1		
Control Delay (s/veh)								9.6	12.4					10.1		
Level of Service (LOS)								A	B					B		
Approach Delay (s/veh)					9.6				0.0				0.2			
Approach LOS					A											

HCS7 Two-Way Stop-Control Report

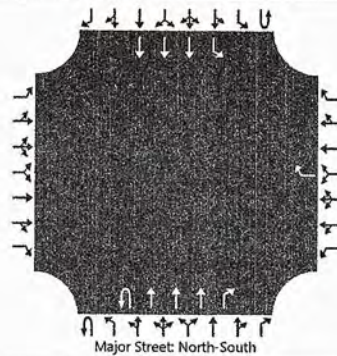
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	3/16/2021
Analysis Year	2024
Time Analyzed	PM
Intersection Orientation	North-South
Project Description	Costco

Site Information

Intersection	Kanner Hwy & Driveway 2
Jurisdiction	Martin County
East/West Street	Driveway 2
North/South Street	Kanner Hwy
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	0	1	1	0	3	1	0	1	3	0
Configuration								R	U		T	R		L	T	
Volume (veh/h)								69	0		1272	53	0	53	1066	
Percent Heavy Vehicles (%)								3	3				3	3		
Proportion Time Blocked								0.250						0.250		
Percent Grade (%)					0											
Right Turn Channelized					No				No							
Median Type Storage					Left Only								1			

Critical and Follow-up Headways

Base Critical Headway (sec)								7.1	5.6						5.3		
Critical Headway (sec)								7.16	5.66						5.36		
Base Follow-Up Headway (sec)								3.3	2.3						3.1		
Follow-Up Headway (sec)								3.33	2.33						3.13		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)							73	0						56		
Capacity, c (veh/h)							811	549						588		
v/c Ratio							0.09	0.00						0.09		
95% Queue Length, Q ₉₅ (veh)							0.3	0.0						0.3		
Control Delay (s/veh)							9.9	11.6						11.8		
Level of Service (LOS)							A	B						B		
Approach Delay (s/veh)							9.9		0.0				0.6			
Approach LOS							A									

TURNING MOVEMENT VOLUME COUNTS

N/S STREET: Willowby Blvd E/W STREET: Driveway 3 CONTROL: TVSC
 FILENAME: 2019 DAY: Thursday CITY: Stuart INTERSECTION:
 COUNT DATE: 1/15/2020 ANALYSIS YEAR: 2024
 REPORT DATE: 1/15/2020

15 Min Period	Northbound				Southbound				Eastbound				Westbound				ONE HOUR SUM
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL	WBL	WBT	WBR	
7:00-7:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15-7:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30-7:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45-8:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00-8:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15-8:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30-8:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45-9:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

AM PEAK HOUR IS FROM: 7:00AM TO 8:00AM

Volumes	0	401	0	0	666	0	0	0	0	0	0	0	0	0	0	0	1057
Season Factor	0	401	0	0	666	0	0	0	0	0	0	0	0	0	0	0	1280
Growth	0	481	0	0	799	0	0	0	0	0	0	0	0	0	0	0	-
In/Out	IN	-	-	IN	OUT	IN	OUT	-	OUT	-	-	-	-	-	-	-	-
Retail Percent	5%	0%	0%	0%	15%	15%	15%	0%	10%	0%	0%	0%	0%	0%	0%	0%	0%
Res. Percent	25%	0%	0%	0%	35%	35%	35%	0%	25%	0%	0%	0%	0%	0%	0%	0%	0%
Retail Trips	12	0	0	0	37	27	0	18	0	0	0	0	0	0	0	0	0
Res. Trips	8	0	0	0	10	23	0	19	0	0	0	0	0	0	0	0	0

Seasonal Factor: 1
 Growth Rate: 1.037
 Years Grown: 5

Total	20	481	0	0	799	47	50	0	37	0	0	0	0	0	0	0	1484
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15 Min Period	Northbound				Southbound				Eastbound				Westbound				ONE HOUR SUM
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	TOTAL	WBL	WBT	WBR	
4:00-4:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15-4:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30-4:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45-5:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00-5:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15-5:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30-5:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45-6:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

PM PEAK HOUR IS FROM: 4:00PM TO 5:00PM

Volumes	0	666	0	0	401	0	0	0	0	0	0	0	0	0	0	0	0
Season Factor	0	666	0	0	401	0	0	0	0	0	0	0	0	0	0	0	1057
Growth	0	799	0	0	481	0	0	0	0	0	0	0	0	0	0	0	1280
In/Out	IN	-	-	IN	OUT	IN	OUT	-	OUT	-	-	-	-	-	-	-	-
Retail Percent	5%	0%	0%	0%	15%	15%	15%	0%	10%	0%	0%	0%	0%	0%	0%	0%	0%
Res. Percent	25%	0%	0%	0%	35%	35%	35%	0%	25%	0%	0%	0%	0%	0%	0%	0%	0%
Retail Trips	12	0	0	0	37	27	0	18	0	0	0	0	0	0	0	0	0
Res. Trips	8	0	0	0	10	23	0	19	0	0	0	0	0	0	0	0	0

Seasonal Factor: 1
 Growth Rate: 1.037
 Years Grown: 5

Total	44	799	0	0	481	97	81	0	56	0	0	0	0	0	0	0	1556
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HCS7 Two-Way Stop-Control Report

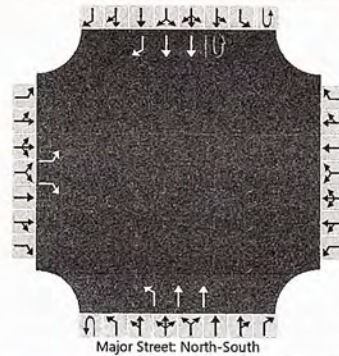
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	4/21/2021
Analysis Year	2024
Time Analyzed	AM Peak Hour
Intersection Orientation	North-South
Project Description	Costco

Site Information

Intersection	Driveway 3 & Willoughby
Jurisdiction	Martin County
East/West Street	Driveway 3
North/South Street	Willoughby Blvd
Peak Hour Factor	0.92
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	0	1		0	0	0	0	1	2	0	1	0	2	1
Configuration		L		R						L	T		U		T	R
Volume (veh/h)		50		37					0	20	481		0		799	47
Percent Heavy Vehicles (%)		3		3					3	3			3			
Proportion Time Blocked																
Percent Grade (%)	0															
Right Turn Channelized	No												No			
Median Type Storage	Left Only								1							

Critical and Follow-up Headways

Base Critical Headway (sec)		7.5		6.9						4.1			6.4			
Critical Headway (sec)		6.86		6.96						4.16			6.46			
Base Follow-Up Headway (sec)		3.5		3.3						2.2			2.5			
Follow-Up Headway (sec)		3.53		3.33						2.23			2.53			

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		54		40						22			0			
Capacity, c (veh/h)		290		567						732			665			
v/c Ratio		0.19		0.07						0.03			0.00			
95% Queue Length, Q ₉₅ (veh)		0.7		0.2						0.1			0.0			
Control Delay (s/veh)		20.3		11.8						10.1			10.4			
Level of Service (LOS)		C		B						B			B			
Approach Delay (s/veh)	16.7								0.4				0.0			
Approach LOS	C															

HCS7 Two-Way Stop-Control Report

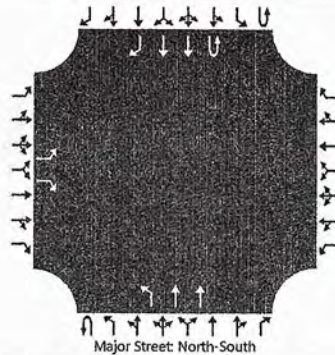
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	3/16/2021
Analysis Year	2024
Time Analyzed	PM Peak Hour
Intersection Orientation	North-South
Project Description	Costco

Site Information

Intersection	Driveway 3 & Willoughby
Jurisdiction	Martin County
East/West Street	Driveway 3
North/South Street	Willoughby Blvd
Peak Hour Factor	0.92
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		1	0	1		0	0	0	0	1	2	0	1	0	2	1
Configuration		L		R						L	T		U		T	R
Volume (veh/h)		81		56					0	44	799		0		481	97
Percent Heavy Vehicles (%)		3		3					3	3			3			
Proportion Time Blocked																
Percent Grade (%)		0														
Right Turn Channelized		No											No			
Median Type Storage		Left Only										1				

Critical and Follow-up Headways

Base Critical Headway (sec)		7.5		6.9						4.1			6.4			
Critical Headway (sec)		6.86		6.96						4.16			6.46			
Base Follow-Up Headway (sec)		3.5		3.3						2.2			2.5			
Follow-Up Headway (sec)		3.53		3.33						2.23			2.53			

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		88		61						48			0			
Capacity, c (veh/h)		341		734						943			400			
v/c Ratio		0.26		0.08						0.05			0.00			
95% Queue Length, Q ₉₅ (veh)		1.0		0.3						0.2			0.0			
Control Delay (s/veh)		19.2		10.3						9.0			14.0			
Level of Service (LOS)		C		B						A			B			
Approach Delay (s/veh)		15.6								0.5				0.0		
Approach LOS		C														

TURNING MOVEMENT VOLUME COUNTS

N/S STREET: Kanner Hwy
 FILENAME: 3/20/2019
 COUNT DATE: 12/16/2020
 DAY: Wednesday
 ANALYSIS YEAR: 2024
 CITY: Stuart
 INTERSECTION: E/W STREET, Driveway 4
 CONTROL:

15 Min Period	Northbound			Southbound			Eastbound			Westbound			ONE HOUR SUM	18 ←	12 →				
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR				TOTAL			
7:00-7:15	0	163	0	0	277	0	0	0	0	0	0	0	440	2040	0	1179	0	↓	↑
7:15-7:30	0	230	0	0	296	0	0	0	0	0	0	0	526	2149	0	1179	0	↓	↑
7:30-7:45	0	254	0	0	253	0	0	0	0	0	0	0	507	2138	0	1179	0	↓	↑
7:45-8:00	0	298	0	0	269	0	0	0	0	0	0	0	567	2127	0	1179	0	↓	↑
8:00-8:15	0	284	0	0	265	0	0	0	0	0	0	0	549	1976	0	1179	0	↓	↑
8:15-8:30	0	329	0	0	186	0	0	0	0	0	0	0	515		0	1149	12	↓	↑
8:30-8:45	0	256	0	0	240	0	0	0	0	0	0	0	496		0	1149	12	↓	↑
8:45-9:00	0	218	0	0	198	0	0	0	0	0	0	0	416		0	1149	12	↓	↑

AM PEAK HOUR IS FROM: 7:15AM TO 8:15AM

Volumes	0	1066	0	0	0	1083	0	0	0	0	0	0	0	2149	
Season Factor	0	1066	0	0	0	1083	0	0	0	0	0	0	0	2149	
Growth	0	1093	0	0	0	1110	0	0	0	0	0	0	0	2203	
In/Out	-	IN	IN	-	OUT	-	-	-	-	-	-	-	-	OUT	-
Retail Percent	0%	20%	5%	0%	30%	0%	0%	0%	0%	0%	0%	0%	10%	0%	0%
Res. Percent	0%	20%	0%	0%	20%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Retail Trips	0	50	12	0	54	0	0	0	0	0	0	0	18	0	0
Res. Trips	0	6	0	0	15	0	0	0	0	0	0	0	0	0	0

Seasonal Factor: 1
 Growth Rate: 1.005
 Years Growth: 5

Total	0	1149	12	0	1179	0	0	0	0	0	0	0	0	18	2359
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15 Min Period lanes	Northbound			Southbound			Eastbound			Westbound			ONE HOUR SUM	
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR		TOTAL
4:00-4:15	0	317	0	0	226	0	0	0	0	0	0	0	543	2010
	0	281	0	0	203	0	0	0	0	0	0	0	484	1993
	0	287	0	0	212	0	0	0	0	0	0	0	499	1984
4:45-5:00	0	240	0	0	244	0	0	0	0	0	0	0	484	1978
	0	275	0	0	251	0	0	0	0	0	0	0	526	2001
5:00-5:15	0	260	0	0	215	0	0	0	0	0	0	0	475	
5:15-5:30	0	271	0	0	222	0	0	0	0	0	0	0	493	
5:30-5:45	0	292	0	0	215	0	0	0	0	0	0	0	507	
5:45-6:00														

PM PEAK HOUR IS FROM: 4:00PM TO 5:00PM

Volumes	0	1125	0	0	0	885	0	0	0	0	0	0	0	2010	
Season Factor	0	1125	0	0	0	885	0	0	0	0	0	0	0	2010	
Growth	0	1153	0	0	0	907	0	0	0	0	0	0	0	2051	
In/Out	-	IN	IN	-	OUT	-	-	-	-	-	-	-	-	OUT	-
Retail Percent	0%	20%	5%	0%	30%	0%	0%	0%	0%	0%	0%	0%	10%	0%	0%
Res. Percent	0%	20%	0%	0%	20%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Retail Trips	0	96	25	0	134	0	0	0	0	0	0	0	45	0	0
Res. Trips	0	25	0	0	9	0	0	0	0	0	0	0	0	0	0

Seasonal Factor: 1
 Growth Rate: 1.005
 Years Growth: 5

Total	0	1267	25	0	1050	0	0	0	0	0	0	0	0	45	2386
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HCS7 Two-Way Stop-Control Report

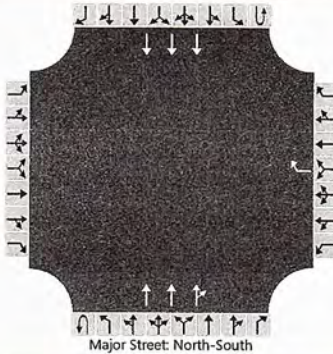
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	4/21/2021
Analysis Year	2024
Time Analyzed	AM
Intersection Orientation	North-South
Project Description	M&M Retail

Site Information

Intersection	Kanner Hwy & Driveway 4
Jurisdiction	Martin County
East/West Street	Driveway 4
North/South Street	Kanner Hwy
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	0	1	0	0	3	0	0	0	3	0
Configuration								R			T	TR			T	
Volume (veh/h)								18			1149	12			1179	
Percent Heavy Vehicles (%)								3								
Proportion Time Blocked																
Percent Grade (%)					0											
Right Turn Channelized					Yes											
Median Type Storage	Left Only								1							

Critical and Follow-up Headways

Base Critical Headway (sec)								7.1								
Critical Headway (sec)								7.16								
Base Follow-Up Headway (sec)								3.3								
Follow-Up Headway (sec)								3.33								

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)								19								
Capacity, c (veh/h)								420								
v/c Ratio								0.05								
95% Queue Length, Q ₉₅ (veh)								0.1								
Control Delay (s/veh)								14.0								
Level of Service (LOS)								B								
Approach Delay (s/veh)					14.0											
Approach LOS					B											

HCS7 Two-Way Stop-Control Report

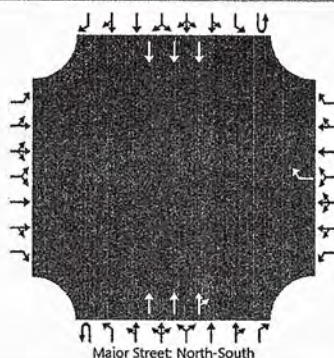
General Information

Analyst	James Kemp
Agency/Co.	O'Rourke Engineering
Date Performed	3/16/2021
Analysis Year	2024
Time Analyzed	PM
Intersection Orientation	North-South
Project Description	M&M Retail

Site Information

Intersection	Kanner Hwy & Driveway 4
Jurisdiction	Martin County
East/West Street	Driveway 4
North/South Street	Kanner Hwy
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	0	1	0	0	3	0	0	0	3	0
Configuration								R			T	TR			T	
Volume (veh/h)								45			1267	25			1050	
Percent Heavy Vehicles (%)								3								
Proportion Time Blocked																
Percent Grade (%)					0											
Right Turn Channelized					No											
Median Type Storage					Left Only								1			

Critical and Follow-up Headways

Base Critical Headway (sec)								7.1								
Critical Headway (sec)								7.16								
Base Follow-Up Headway (sec)								3.3								
Follow-Up Headway (sec)								3.33								

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)								47								
Capacity, c (veh/h)								377								
v/c Ratio								0.13								
95% Queue Length, Q ₉₅ (veh)								0.4								
Control Delay (s/veh)								15.9								
Level of Service (LOS)								C								
Approach Delay (s/veh)					15.9											
Approach LOS					C											



O'ROURKE
ENGINEERING & PLANNING

LAND USE PLAN AMENDMENT

TRAFFIC ANALYSIS

Prepared for:

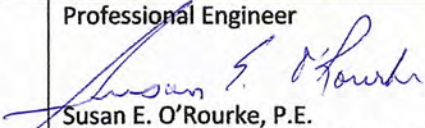
Mr. Joe Marino
M & M Retail Partners, LLC.
Lodi, NJ 07644

Prepared by:

O'Rourke Engineering & Planning
22 SE Seminole Street
Stuart, Florida 34994
(772) 781-7918

December 18, 2020
Revised March 16, 2021
Revised April 21, 2021

MR18031.0

Prepared by: O'Rourke Engineering & Planning Certificate of Authorization: #26869 22 SE Seminole Street Stuart, Florida 34994 772-781-7918	Professional Engineer  Susan E. O'Rourke, P.E. Date signed and sealed: 4/21/2021 License #: 42684
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O'ROURKE
ENGINEERING & PLANNING

December 18, 2020

Mr. Joe Marino
M & M Retail Partners, LLC.
Lodi, NJ 07644

Re: Land Use Plan Amendment Traffic Analysis

Dear Mr. Marino:

O'Rourke Engineering & Planning has completed the traffic analysis of the proposed land use plan amendment for a change of 48.97 acres from Low Density Residential to a Neighborhood Special District category. The parcels affected by the land use plan amendment are located on SR-76 (Kanner Highway) in the city of Stuart, Martin County, Florida. The steps in the analysis and the ensuing results are presented herein.

It has been a pleasure working with you, if you have any questions or comments, please do not hesitate to contact our office.

Respectfully submitted,
O'ROURKE ENGINEERING & PLANNING

Susan E. O'Rourke, P.E.
Registered Civil Engineer – Traffic

C6 – LUPA Report – Costco – 4.21.21

TABLE OF CONTENTS

INTRODUCTION	1
PROJECT DESCRIPTION	1
ROADWAY NETWORK	3
MAXIMUM ALLOWABLE USE / TRIP GENERATION	4
PROJECT ASSIGNMENT	8
STUDY AREA	8
LONG RANGE ANALYSIS - 2045	8
FIVE YEAR ANALYSIS – 2025	12
CONCLUSION	12

TABLES

TABLE 1: Trip Generation – Existing Future Land Use	5
TABLE 2: Trip Generation – Proposed Future Land Use	6
TABLE 3: Proposed FLU – Existing FLU	7
TABLE 4: Percent Impact 2045 AM	10
TABLE 5: Link Analysis 2045	11
TABLE 6: Percent Impact 2025	13
TABLE 7: Link Analysis 2025	14

FIGURES

FIGURE 1: Project Location	2
FIGURE 2a: Percent Assignment – Retail	9
FIGURE 2b: Percent Assignment – Residential	10

APPENDICES

APPENDIX A: Roadway Network Data/Non-Motorized and Transit Data	
APPENDIX B: Internal Capture Matrices / Pass-By Calculation	
APPENDIX C: Martin County 2019 Roadway Level of Service Inventory Report and Martin County 2040 Roadway Level of Service Inventory Report	

INTRODUCTION

O'Rourke Engineering & Planning was retained to prepare a traffic analysis for the proposed land use plan amendment of 48.97-acres. The following components were addressed:

- Summary of the project description; existing land use and proposed land use
- Summary of road network, existing and 2045
- Assessment of the change in trip generation
- Summary of 2025 traffic volumes
- Assessment of net change in 2045 impact

Each of these components is outlined herein.

PROJECT DESCRIPTION

The proposed land use plan amendment involves parcels of land located on SR-76 (Kanner Highway) in the city of Stuart, Martin County, Florida. The project location and lanes are shown in **Figure 1**.

The existing future land use designation of the 48.97-acres was previously Low Density Residential in the Martin County Future Land Use Map. The parcels were annexed by the City of Stuart on January 17, 2017. The future land use of the parcels has not been designated by the City of Stuart, therefore, the previous Existing Land Use from Martin County was considered in the analysis. Under the existing future land use a maximum of 5 dwelling units per acre is allowed, allowing for a total of 244 single-family dwelling units.

The future land use would be amended to Neighborhood Special District allowing 398 multi-family dwelling units and 193,021 square feet of Non-Residential Development.

The analysis of the project impacts in the five-year period and the long-range scenario, 2045 are discussed herein.



Figure 1
Project Location
M&M Retail

Legend

- = Signalized Intersection
- = Project Location
- 4LD = Roadway Laneage (4LD) = 2045 Lanes



22 SE Seminole St
Stuart, FL 34994

Date: 12/18/20



NTS

Job Number: MR18031.0

ROADWAY NETWORK

The study area was reviewed to determine the existing number and type of lanes, and the traffic control along the roadway. Each roadway is described below.

SR-714 (Monterey Road/Palm City Bridge) is a four-lane divided major arterial with a general east/west alignment. From Matheson to Palm City Road is included in the Cost Feasible Plan for 6 laning.

Indian Street is a four-lane divided major arterial with a primarily east/west alignment.

SR 76 (Kanner Highway) is a six- lane, divided major arterial from I-95 to US-1. It has a generally east/west alignment from west of I-95 to I-95 and then travels in a generally north/south alignment.

CR-714 (Veteran's Memorial Bridge) is a four-lane divided major arterial with a primarily east/west alignment.

Appendix A includes the roadway network information to include the 2045 LRTP Cost Feasible Network and the non-motorized transit facilities.

MAXIMUM ALLOWABLE USE/TRIP GENERATION

To determine the worst-case scenario from a traffic standpoint, the trip generation for allowable uses under each existing and future land use were calculated. The Existing Future Land Use is Low Density Residential which allows for up to 244 single-family dwelling units. The trip generation for the existing future land use is shown in **Table 1a, 1b, and 1c** for the daily, AM peak hour and PM peak hours, respectively.

The Proposed Future Land Use will be Neighborhood Special District. Under this land use there would be a cap limiting development of 193,021 square feet of retail or equivalent and 398 multi-family dwelling units. **Table 2a, 2b, and 2c** summarizes the trip generation for the proposed future land use for the daily, AM peak hour and PM peak hours, respectively. The internal capture matrices for the proposed future land use are included in **Appendix B. Table 3** summarizes the net change in trips.

As shown, the existing future land use generates 2,362 daily trips; 178 AM Peak hour trips and 239 PM Peak hour trips. Under the proposed future land use, the site generates 7,410 daily trips; 341 AM Peak hour trips and 668 PM peak hour trips. The change in trips between the Proposed and Existing future land uses is an increase of 5,048 daily trips; 163 AM peak hour trips and 429 PM peak hour trips.

Table 1 - Trip Generation - Existing FLU

Table 1a: Daily

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Net New Trips		
					In	Out	In	Out	Total
Single Family Detached	210	244	DU	$\text{Ln}(T)=0.92\text{Ln}(X)+2.71$	50%	50%	1,181	1,181	2,362
TOTALS							1,181	1,181	2,362

Source: ITE 10th Edition Trip Generation Rates

Table 1b: AM Peak Hour

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Net New Trips		
					In	Out	In	Out	Total
Single Family Detached	210	244	DU	$T=0.71(X)+4.80$	25%	75%	45	133	178
TOTALS							45	133	178

Source: ITE 10th Edition Trip Generation Rates

Table 1c: PM Peak Hour

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Net New Trips		
					In	Out	In	Out	Total
Single Family Detached	210	244	DU	$\text{Ln}(T)=0.96\text{Ln}(X)+0.20$	63%	37%	151	88	239
TOTALS							151	88	239

Source: ITE 10th Edition Trip Generation Rates

Table 2 - Trip Generation - Proposed FLU

Table 2a: Daily

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips		Net External Trips		Pass-by Trips		Net New Trips	
					In	Out	In	Out	In	Out	In	Out	In	Out	In	Out
Multifamily Housing (Mid-Rise)	221	398	DJ	$T = 5.45(X) \cdot 1.75$	50%	50%	1,084	1,083	2,167	76	228	304	14,09%	1,863	855	1,863
Shopping Center	820	16,750	SR	$\text{Ln}(T) = 0.68 \cdot \text{Ln}(X) + 5.57$	50%	50%	892	892	1,784	47	121	168	9.4%	845	771	1,616
Discount Club	857	157,531	SR	$T = 41.80(X)$	50%	50%	3,293	3,292	6,585	174	445	619	9.4%	3,119	2,847	5,966
Fast Food (w/ Drive Thru)	934	2,500	SR	$T = 470.95(X)$	50%	50%	589	588	1,177	299	104	403	34.3%	290	484	774
High-Turnover (Sit-Down) Restaurant	932	16,240	SR	$T = 112.18(X)$	50%	50%	911	911	1,822	462	160	622	34.1%	449	751	1,200
Retail Sub Total							5,685	5,683	11,368	982	830	1,812	15.6%	4,703	4,853	9,556
TOTALS		193,021					6,769	6,766	13,535	1,058	1,058	2,116		5,711	5,708	11,419

Source: ITE 10th Edition Trip Generation Rates
Total 162,820 w/ Outdoor/Ventilable 4,489

Table 2b: AM Peak Hour

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips		Net External Trips		Pass-by Trips		Net New Trips	
					In	Out	In	Out	In	Out	In	Out	In	Out	In	Out
Multifamily Housing (Mid-Rise)	221	398	DJ	$\text{Ln}(T) = 0.98 \cdot \text{Ln}(X) - 0.98$	26%	74%	35	98	133	3	21	24	18.0%	32	77	109
Shopping Center	820	16,750	SR	$T = 0.50(X) + 151.78$	62%	38%	99	61	160	9	8	17	10.0%	90	53	143
Discount Club	857	157,531	SR	$T = 0.49(X)$	70%	30%	54	23	77	4	4	8	10.0%	50	19	69
Fast Food (w/ Drive Thru)	934	2,500	SR	$T = 40.19(X)$	51%	49%	51	49	100	12	5	17	17.0%	39	44	83
High-Turnover (Sit-Down) Restaurant	932	16,240	SR	$T = 9.94(X)$	55%	45%	89	72	161	19	9	28	17.4%	70	63	133
Retail Sub Total							293	205	498	44	26	70	14.1%	249	179	428
TOTALS		193,021					328	303	631	47	47	94	14.9%	281	256	537

Source: ITE 10th Edition Trip Generation Rates

Table 2c: PM Peak Hour

Land Use	ITE Code	Intensity	Units	Trip Generation Rate	Directional Split		Gross Trips		Internalization Trips (1)		Net External Trips		Pass-by Trips		Net New Trips	
					In	Out	In	Out	In	Out	In	Out	In	Out	In	Out
Multifamily Housing (Mid-Rise)	221	398	DJ	$\text{Ln}(T) = 0.96 \cdot \text{Ln}(X) - 0.63$	61%	39%	102	65	167	25	20	45	26.9%	77	45	122
Shopping Center	820	16,750	SR	$\text{Ln}(T) = 0.74 \cdot \text{Ln}(X) + 2.89$	48%	52%	70	75	145	4	7	11	7.6%	66	134	32
Discount Club	857	157,531	SR	$T = 4.18(X)$	50%	50%	329	329	658	19	29	48	7.3%	310	300	610
Fast Food (w/ Drive Thru)	934	2,500	SR	$T = 32.67(X)$	52%	48%	43	39	82	10	7	17	20.7%	33	32	65
High-Turnover (Sit-Down) Restaurant	932	16,240	SR	$T = 9.77(X)$	62%	38%	99	60	159	18	13	31	19.5%	81	47	128
Retail Sub Total							541	503	1,044	51	56	107	10.2%	490	447	937
TOTALS		193,021					643	568	1,211	76	152	12.6%	567	492	1,059	567

Source: ITE 10th Edition Trip Generation Rates
(1) ITE Pairings reduced, Residential to Retail from 43 to 20 and Retail to Residential from 46 to 15, Residential to Restaurant from 21 to 10 and Restaurant to Residential from 18 to 10, Restaurant to Retail from 41 to 10 and Retail to Restaurant from 29 to 15 to avoid overstating internal capture for PM Peak Hour and Daily

Table 3 Trip Generation Proposed FLU - Existing FLU

Table 3a- Daily

Description	Gross Trips			Daily Net New Trips		
	In	Out	Total	In	Out	Total
Proposed FLU	6,769	6,766	13,535	3,706	3,704	7,410
Existing FLU	1,181	1,181	2,362	1,181	1,181	2,362
Net Change	5,588	5,585	11,173	2,525	2,523	5,048

Table 3b- AM Peak Hour

Description	Gross Trips			AM Net New Trips		
	In	Out	Total	In	Out	Total
Proposed FLU	328	303	631	181	160	341
Existing FLU	45	133	178	45	133	178
Net Change	283	170	453	136	27	163

Table 3c - PM Peak Hour

Description	Gross Trips			PM Net New Trips		
	In	Out	Total	In	Out	Total
Proposed FLU	643	568	1,211	372	296	668
Existing FLU	151	88	239	151	88	239
Net Change	492	480	972	221	208	429

PROJECT ASSIGNMENT

The project traffic was distributed by general geographic direction and then assigned to the roadway network. This general distribution led to an assignment of trips based on the anticipated ultimate destinations and the roadway paths used to reach those destinations. **Figure 2a** and **Figure 2b** show the project assignment for the retail and residential uses, respectively.

STUDY AREA

The study area was defined as the area upon which the project traffic (the net increase in traffic) represented 5% or more on the roadway link, the adjacent links and any currently failing link. **Table 4a** and **4b** summarize the project % impact on the 2045 long range cost feasible roadway network.

LONG RANGE ANALYSIS – 2040/2045

The 2045 LRTP has been adopted. However, 2045 future traffic volumes have not been available. Therefore, the 2045 volumes were used as the best available for this analysis. To determine the impact of the change in traffic for the 2045 long term analysis, the projected 2040 daily volume from the Martin County 2040 Roadway Level of Service Inventory Report were converted to peak hour directional volumes using the K and D factors obtained from the FDOT Florida Traffic Online Historical AADT for Kanner Highway (site 0114) and Willoughby Boulevard (site 0063). The project traffic was then added to the peak hour directional volumes and compared to the cost feasible service capacities to determine the impacts of the land use plan amendment. As shown, all roads will operate at an acceptable level of service with the 2045 long range cost feasible network. **Table 5** includes the 2040/2045 link analysis.

Appendix C includes the Martin County 2040 Roadway Level of Service Inventory Report.

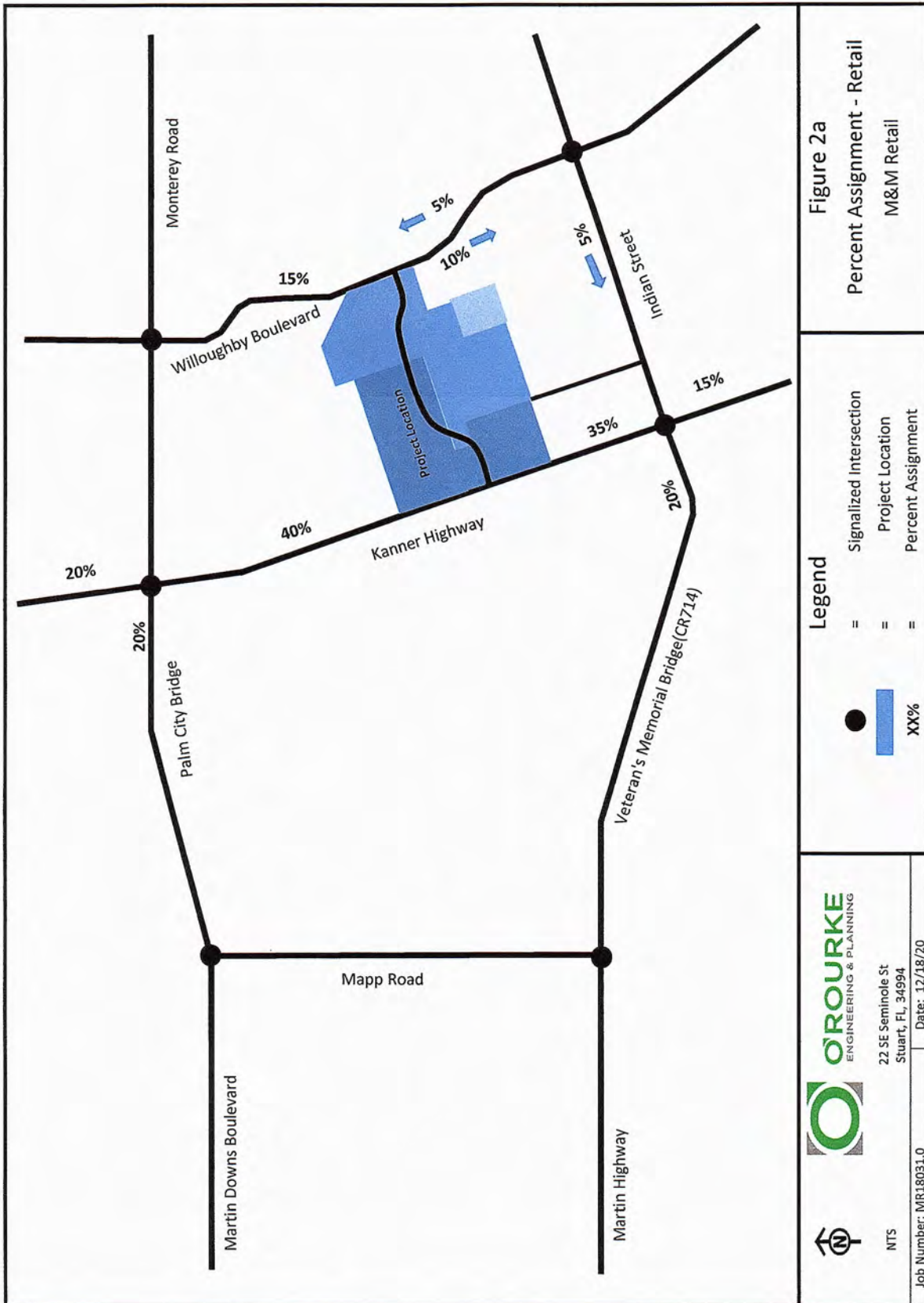


Table 4a: Percent Impact - AM Peak Hour - 2045

Segment	From	To	Direction		Lanes	Is Project Traffic 5% or More of Capacity?	LOS D Directional Peak Hour	Directional Peak Project Volume	Project Percent Assignment	Directional Percent Project of Capacity
SR-76 (Kamer Hwy)	Salerno Rd	Indian St	NB	IN	6LD	no	3,020	20	15%	0.66%
	Salerno Rd	Indian St	SB	OUT	6LD	no	3,020	4	15%	0.13%
	Indian St	Project Entrance	NB	IN	6LD	no	3,020	61	45%	2.02%
	Indian St	Project Entrance	SB	OUT	6LD	no	3,020	12	45%	0.40%
	Project Entrance	SR-714	NB	OUT	6LD	no	3,020	15	55%	0.50%
	Project Entrance	SR-714	SB	IN	6LD	no	3,020	75	55%	2.48%
	SR-714	SR-5	NB	OUT	6LD	no	3,020	5	20%	0.17%
	SR-714	SR-5	SB	IN	6LD	no	3,020	27	20%	0.89%
	Indian St	Project Entrance	NB	IN	4LD	no	2,000	34	25%	1.70%
	Indian St	Project Entrance	SB	OUT	4LD	no	2,000	7	25%	0.35%
SR-714 *	Project Entrance	SR-714	NB	OUT	4LD	no	2,000	8	30%	0.40%
	Project Entrance	SR-714	SB	IN	4LD	no	2,000	41	30%	2.05%
	Mapp Rd	SR-76	EB	IN	4LD	no	2,000	27	20%	1.35%
	Mapp Rd	SR-76	WB	OUT	4LD	no	2,000	5	20%	0.25%
SR-714 (Monterey Rd)	SR-76	Willoughby Blvd	EB	OUT	4LD	no	2,000	4	15%	0.20%
	SR-76	Willoughby Blvd	WB	IN	4LD	no	2,000	20	15%	1.00%
Indian St	SR-76	Willoughby Blvd	EB	OUT	4LD	no	2,000	3	10%	0.15%
	SR-76	Willoughby Blvd	WB	IN	4LD	no	2,000	14	10%	0.70%
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	EB	IN	4LD	no	2,000	27	20%	1.35%
	Mapp Rd	SR-76	WB	OUT	4LD	no	2,000	5	20%	0.25%

* Note: Mapp Road to Palm City Road included as 6LD in LRTP

In: 136
Out: 27

Table 4b: Percent Impact - PM Peak Hour - 2045

Segment	From	To	Direction		Lanes	Is Project Traffic 5% or More of Capacity?	LOS D Directional Peak Hour	Directional Peak Project Volume	Project Percent Assignment	Directional Percent Project of Capacity
SR-76 (Kamer Hwy)	Salerno Rd	Indian St	NB	IN	6LD	no	3,020	33	15%	1.09%
	Salerno Rd	Indian St	SB	OUT	6LD	no	3,020	31	15%	1.03%
	Indian St	Project Entrance	NB	IN	6LD	no	3,020	99	45%	3.28%
	Indian St	Project Entrance	SB	OUT	6LD	no	3,020	94	45%	3.11%
	Project Entrance	SR-714	NB	OUT	6LD	no	3,020	114	55%	3.77%
	Project Entrance	SR-714	SB	IN	6LD	no	3,020	122	55%	4.04%
	SR-714	SR-5	NB	OUT	6LD	no	3,020	42	20%	1.39%
	SR-714	SR-5	SB	IN	6LD	no	3,020	44	20%	1.46%
	Indian St	Project Entrance	NB	IN	4LD	no	2,000	55	25%	2.75%
	Indian St	Project Entrance	SB	OUT	4LD	no	2,000	52	25%	2.60%
SR-714 *	Project Entrance	SR-714	NB	OUT	4LD	no	2,000	62	30%	3.10%
	Project Entrance	SR-714	SB	IN	4LD	no	2,000	66	30%	3.30%
	Mapp Rd	SR-76	EB	IN	4LD	no	2,000	44	20%	2.20%
	Mapp Rd	SR-76	WB	OUT	4LD	no	2,000	42	20%	2.10%
SR-714 (Monterey Rd)	SR-76	Willoughby Blvd	EB	OUT	4LD	no	2,000	31	15%	1.55%
	SR-76	Willoughby Blvd	WB	IN	4LD	no	2,000	33	15%	1.65%
Indian St	SR-76	Willoughby Blvd	EB	OUT	4LD	no	2,000	21	10%	1.05%
	SR-76	Willoughby Blvd	WB	IN	4LD	no	2,000	22	10%	1.10%
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	EB	IN	4LD	no	2,000	44	20%	2.20%
	Mapp Rd	SR-76	WB	OUT	4LD	no	2,000	42	20%	2.10%

* Note: Mapp Road to Palm City Road included as 6LD in LRTP

In: 221
Out: 208

Table 5a: Link Analysis - AM Peak Hour - 2045

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	2040 Model AADT	(1) K Factor	(1) D Factor	2040 Volume Peak Hour Peak Direction	LOS D Directional Peak Hour	Directional Peak Project Volume	Project Percent Assignment	Directional Percent Project of Capacity	Total Traffic (Peak Direction)	Does Project Meet Concurrency?
SR-76 (Kanner Hwy)	Indian St	Project Entrance	NB	6LD	no	35510	0.09	0.376	1202	3020	61	45%	2.02%	1263	Yes
	Indian St	Project Entrance	SB	6LD	no	35510	0.09	0.624	1994	3020	12	45%	0.40%	2006	Yes
	Project Entrance	SR-714	NB	6LD	no	35510	0.09	0.376	1202	3020	15	55%	0.50%	1217	Yes
	Project Entrance	SR-714	SB	6LD	no	35510	0.09	0.624	1994	3020	75	55%	2.48%	2069	Yes
Willoughby Blvd	Indian St	Project Entrance	NB	4LD	no	13616	0.09	0.376	613	2000	34	25%	1.70%	647	Yes
	Indian St	Project Entrance	SB	4LD	no	13616	0.09	0.624	613	2000	7	25%	0.35%	620	Yes
	Project Entrance	SR-714	NB	4LD	no	13390	0.09	0.376	554	2000	8	30%	0.40%	562	Yes
	Project Entrance	SR-714	SB	4LD	no	13390	0.09	0.624	551	2000	41	30%	2.05%	692	Yes

Source: Martin County 2040 Roadway Level of Service Inventory Report

(1) FDOT Florida Traffic Online (Station 0114 and 0063)

In: 136

Out: 27

Table 5b: Link Analysis - PM Peak Hour - 2045

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	2040 Model AADT	(1) K Factor	(1) D Factor	2040 Volume Peak Hour Peak Direction	LOS D Directional Peak Hour	Directional Peak Project Volume	Project Percent Assignment	Directional Percent Project of Capacity	Total Traffic (Peak Direction)	Does Project Meet Concurrency?
SR-76 (Kanner Hwy)	Indian St	Project Entrance	NB	6LD	no	35510	0.09	0.624	1994	3020	99	45%	3.28%	2093	Yes
	Indian St	Project Entrance	SB	6LD	no	35510	0.09	0.376	1202	3020	94	45%	3.11%	1296	Yes
	Project Entrance	SR-714	NB	6LD	no	35510	0.09	0.624	1994	3020	114	55%	3.77%	2108	Yes
	Project Entrance	SR-714	SB	6LD	no	35510	0.09	0.376	1202	3020	122	55%	4.04%	1324	Yes
Willoughby Blvd	Indian St	Project Entrance	NB	4LD	no	13616	0.09	0.624	613	2000	55	25%	2.75%	668	Yes
	Indian St	Project Entrance	SB	4LD	no	13616	0.09	0.376	613	2000	52	25%	2.60%	665	Yes
	Project Entrance	SR-714	NB	4LD	no	13390	0.09	0.624	551	2000	62	30%	3.10%	713	Yes
	Project Entrance	SR-714	SB	4LD	no	13390	0.09	0.376	554	2000	66	30%	3.30%	620	Yes

Source: Martin County 2040 Roadway Level of Service Inventory Report

(1) FDOT Florida Traffic Online (Station 0114 and 0063)

In: 221

Out: 208

FIVE YEAR ANALYSIS - 2025

For M&M Retail, the entire 193,021 square feet of non-residential and 398 multi-family dwelling units could be completed in five-years. The trip generation for the 5-year analysis as shown in Table 2a, 2b, and 2c for the daily, AM peak hour and PM peak hours, respectively. As shown, the project would generate 7,410 Daily trips, 341 AM peak hour trips, and 668 PM peak hour trips in the first five years of development.

The purpose of the 5-year analysis is to demonstrate that concurrency could be obtained. To determine the study area for the 5-year analysis, the 5-year project traffic was compared to the existing plus committed roadway network. The study area is defined as the area upon which the project traffic represented 5% or more on the roadway link, the adjacent links and any currently failing link. **Table 6a** and **6b** summarize the project % impact on the existing plus committed roadway network.

To develop total traffic estimates for 2025, existing 2019 traffic was grown to 2025 using historic growth rates. The project traffic was then added to achieve the 2025 total traffic volumes. These volumes were then compared to the capacity of the roadway network (existing plus committed network). The 2019 Martin County Roadway Level of Service Inventory Report was used as the source of the existing 2019 AADT, peak hour data and growth rate. **Table 7a** and **Table 7b** show the link analysis for the total traffic conditions with the proposed land use plan amendment in place in 2025. **Appendix C** provides the Martin County 2019 Roadway Level of Service Inventory Report.

As shown all links will operate at acceptable levels of service on the existing plus committed roadway network.

CONCLUSION

The proposed land use plan amendment will result in an increase in trips of 5,048 daily trips; 163 AM peak hour trips and 429 PM peak hour trips over the Existing Future Land Use.

On the links within the study area, there would be an increase in traffic associated with the land use plan amendment over what could be developed with the existing future land use. The analysis demonstrates that the roadway has sufficient infrastructure funded in the next five years and the 2040/2045 long range to support the project. Therefore, the project satisfies the requirements for a Land Use Plan Amendment.

Table 6a: Percent Impact - AM Peak Hour

Table 6a. Percent Impact - All Year Total												
Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	LOS D Directional Peak Hour	Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume	Total Project Traffic	Directional Percent Project of Capacity
Kanner Highway (SR-76)	Salerno Rd	Indian St	NB	6LD	no	3,020	15%	22	30%	10	32	1.06%
	Salerno Rd	Indian St	SB	6LD	no	3,020	15%	12	30%	23	35	1.16%
	Indian St	Project Entrance	NB	6LD	no	3,020	35%	52	25%	8	60	1.99%
	Indian St	Project Entrance	SB	6LD	no	3,020	35%	29	25%	19	48	1.59%
	Project Entrance	SR-714	NB	6LD	no	3,020	40%	33	20%	15	48	1.59%
	Project Entrance	SR-714	SB	6LD	no	3,020	40%	60	20%	6	66	2.19%
	SR-714	SR-5	NB	6LD	no	3,020	20%	17	30%	23	40	1.32%
	SR-714	SR-5	SB	6LD	no	3,020	20%	30	30%	10	40	1.32%
Willoughby Blvd	Salerno Rd	Pomeroy St	NB	2L	no	880	5%	7	5%	2	9	1.02%
	Salerno Rd	Pomeroy St	SB	2L	no	880	5%	4	5%	4	8	0.91%
	Pomeroy St	Indian St	NB	4LD	no	2,000	5%	7	5%	2	9	0.45%
	Pomeroy St	Indian St	SB	4LD	no	2,000	5%	4	5%	4	8	0.40%
	Indian St	Project Entrance	NB	4LD	no	2,000	5%	7	25%	8	15	0.75%
	Indian St	Project Entrance	SB	4LD	no	2,000	10%	8	25%	19	27	1.35%
	Project Entrance	SR-714	NB	4LD	no	2,000	15%	12	30%	23	35	1.75%
	Project Entrance	SR-714	SB	4LD	no	2,000	15%	22	30%	10	32	1.60%
	SR-714 (Monterey Rd)	Willoughby Blvd	EB	4LD	no	2,000	0%	0	10%	8	8	0.40%
	SR-76	Willoughby Blvd	WB	4LD	no	2,000	0%	0	10%	3	3	0.15%
Indian St	SR-76	Willoughby Blvd	EB	4LD	no	2,000	0%	0	15%	12	12	0.60%
	SR-76	Willoughby Blvd	WB	4LD	no	2,000	5%	7	15%	5	12	0.60%
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	EB	4LD	no	2,000	20%	30	10%	3	33	1.65%
	Mapp Rd	SR-76	WB	4LD	no	2,000	20%	17	10%	8	25	1.25%

Source: Martin County 2019 Roadway Level of Service Inventory Report

Retail	IN	OUT
Residential	149	83
	32	77

Table 6b Percent Impact - PM Peak Hour

Table 66 Percent Impact - PM Peak Hour												
Segment	From	To	Direction		Lanes	Is Project Traffic 5% or More of Capacity?	LOS D Directional Peak Hour	Project Percent Assignment	Directional Peak Project Volume	Project Percent Assignment	Directional Peak Project Volume	Directional Percent Project of Capacity
Kanner Highway (SR-76)	Salerno Rd	Indian St	NB	IN	6LD	no	3,020	15%	44	30%	23	2.22%
	Salerno Rd	Indian St	SB	OUT	6LD	no	3,020	15%	38	30%	14	1.72%
	Indian St	Project Entrance	NB	IN	6LD	no	3,020	35%	103	35%	19	4.04%
	Indian St	Project Entrance	SB	OUT	6LD	no	3,020	35%	88	25%	11	99
	Project Entrance	SR-714	NB	OUT	6LD	no	3,020	40%	100	20%	9	3.61%
	Project Entrance	SR-714	SB	IN	6LD	no	3,020	40%	118	20%	15	4.40%
	SR-714	SR-5	NB	OUT	6LD	no	3,020	20%	50	30%	14	64
	SR-714	SR-5	SB	IN	6LD	no	3,020	20%	59	30%	23	82
Willoughby Blvd	Salerno Rd	Pomeroy St	NB	IN	2L	no	880	5%	15	5%	4	2.16%
	Salerno Rd	Pomeroy St	SB	OUT	2L	no	880	5%	13	5%	2	1.70%
	Pomeroy St	Indian St	NB	IN	4LD	no	2,000	5%	15	5%	4	1.70%
	Pomeroy St	Indian St	SB	OUT	4LD	no	2,000	5%	13	5%	2	1.5
SR-714 (Monterey Rd)	Indian St	Project Entrance	NB	IN	4LD	no	2,000	5%	15	25%	19	34
	Indian St	Project Entrance	SB	OUT	4LD	no	2,000	10%	25	25%	11	36
	Project Entrance	SR-714	NB	OUT	4LD	no	2,000	15%	38	30%	14	52
	Project Entrance	SR-714	SB	IN	4LD	no	2,000	15%	44	30%	23	67
Indian St	SR-76	Willoughby Blvd	EB	OUT	4LD	no	2,000	0%	0	15%	7	7
	SR-76	Willoughby Blvd	WB	IN	4LD	no	2,000	0%	0	15%	12	12
CR-714 (Veteran's Memorial Bridge)	SR-76	Willoughby Blvd	EB	OUT	4LD	no	2,000	5%	15	15%	12	27
	Mapp Rd	SR-76	EB	IN	4LD	no	2,000	20%	59	10%	8	67
	Mapp Rd	SR-76	WB	OUT	4LD	no	2,000	20%	50	10%	5	55

Source: Martin County 2019 Roadway Level of Service Inventory Report

Retail	IN	OUT
Residential	295	251
	77	45

Table 7a: Link Analysis - AM Peak Hour - 2025

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	2019 AADT	D Factor (1)	2019 Volume Peak Hour Peak Direction	Growth Rate	2025 Volume Peak Hour Peak Direction	LOS D Directional Peak Hour	Project Percent Assignment	Directional Peak Project Volume	Residential	Total Project Traffic	Directional Percent Project of Capacity	2025 Total Traffic (Peak Direction)	Does Project Meet Concurrency?
Kanner Highway (SR-76)	Indian St	Project Entrance	NB	IN	6LD	no	21,116	0.376	598	1.005	617	35%	52	25%	60	1.99%	677	Yes
	Indian St	Project Entrance	SB	OUT	6LD	no	21,116	0.624	993	1.005	1023	35%	29	25%	48	1.59%	1071	Yes
	Project Entrance	SR-714	NB	OUT	6LD	no	21,116	0.376	598	1.005	617	40%	33	20%	15	1.59%	665	Yes
	Project Entrance	SR-714	SB	IN	6LD	no	21,116	0.624	993	1.005	1023	40%	60	20%	66	2.19%	1089	Yes
Willoughby Blvd	Indian St	Project Entrance	NB	IN	4LD	no	11,100	0.376	401	1.037	499	5%	8	25%	16	0.75%	515	Yes
	Indian St	Project Entrance	SB	OUT	4LD	no	11,100	0.624	666	1.037	828	10%	12	25%	31	1.35%	859	Yes
	Project Entrance	SR-714	NB	OUT	4LD	no	11,100	0.376	401	1.037	499	15%	12	30%	35	1.75%	534	Yes
	Project Entrance	SR-714	SB	IN	4LD	no	11,100	0.624	666	1.037	828	15%	22	30%	32	1.60%	860	Yes

Source: Martin County 2019 Roadway Level of Service Inventory Report

(1) FDOT Florida Traffic Online

Retail	IN	OUT
Residential	149	83
	32	77

Years Grown: 6

Table 7b: Link Analysis - PM Peak Hour - 2025

Segment	From	To	Direction	Lanes	Is Project Traffic 5% or More of Capacity?	2019 AADT	D Factor (1)	2019 Volume Peak Hour Peak Direction	Growth Rate	2025 Volume Peak Hour Peak Direction	LOS D Directional Peak Hour	Project Percent Assignment	Directional Peak Project Volume	Residential	Total Project Traffic	Directional Percent Project of Capacity	2025 Total Traffic (Peak Direction)	Does Project Meet Concurrency?
Kanner Highway (SR-76)	Indian St	Project Entrance	NB	IN	6LD	no	21,116	0.624	993	1.005	1023	35%	103	25%	122	4.04%	1145	Yes
	Indian St	Project Entrance	SB	OUT	6LD	no	21,116	0.376	598	1.005	617	35%	88	25%	99	3.28%	716	Yes
	Project Entrance	SR-714	NB	OUT	6LD	no	21,116	0.624	993	1.005	1023	40%	100	20%	109	3.61%	1132	Yes
	Project Entrance	SR-714	SB	IN	6LD	no	21,116	0.376	598	1.005	617	40%	118	20%	133	4.40%	750	Yes
Willoughby Blvd	Indian St	Project Entrance	NB	IN	4LD	no	11,100	0.624	666	1.037	828	5%	15	25%	34	1.70%	862	Yes
	Indian St	Project Entrance	SB	OUT	4LD	no	11,100	0.376	401	1.037	499	10%	25	25%	36	1.80%	535	Yes
	Project Entrance	SR-714	NB	OUT	4LD	no	11,100	0.624	666	1.037	828	15%	36	30%	52	2.60%	880	Yes
	Project Entrance	SR-714	SB	IN	4LD	no	11,100	0.376	401	1.037	499	15%	44	30%	67	3.35%	566	Yes

Source: Martin County 2019 Roadway Level of Service Inventory Report

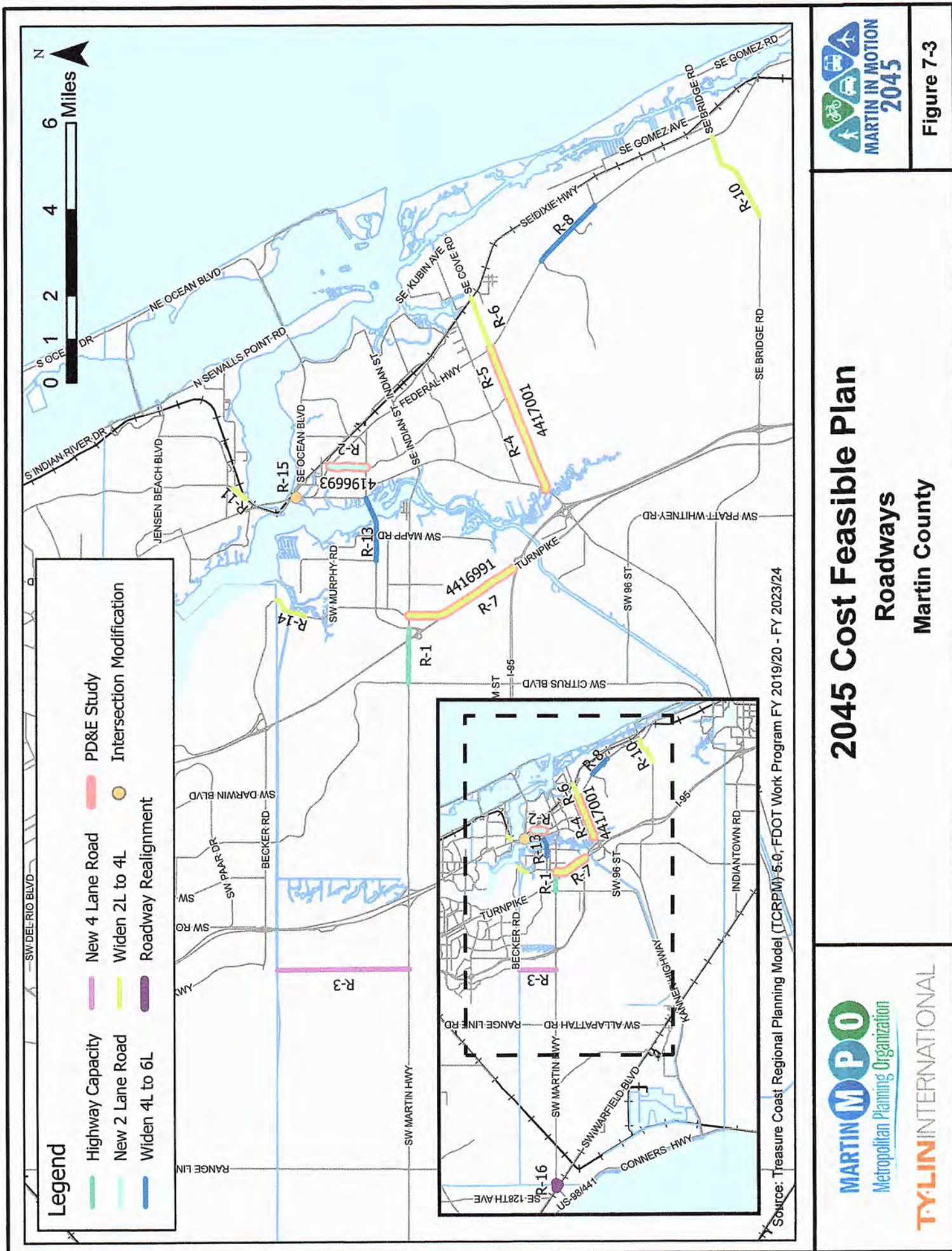
(1) FDOT Florida Traffic Online

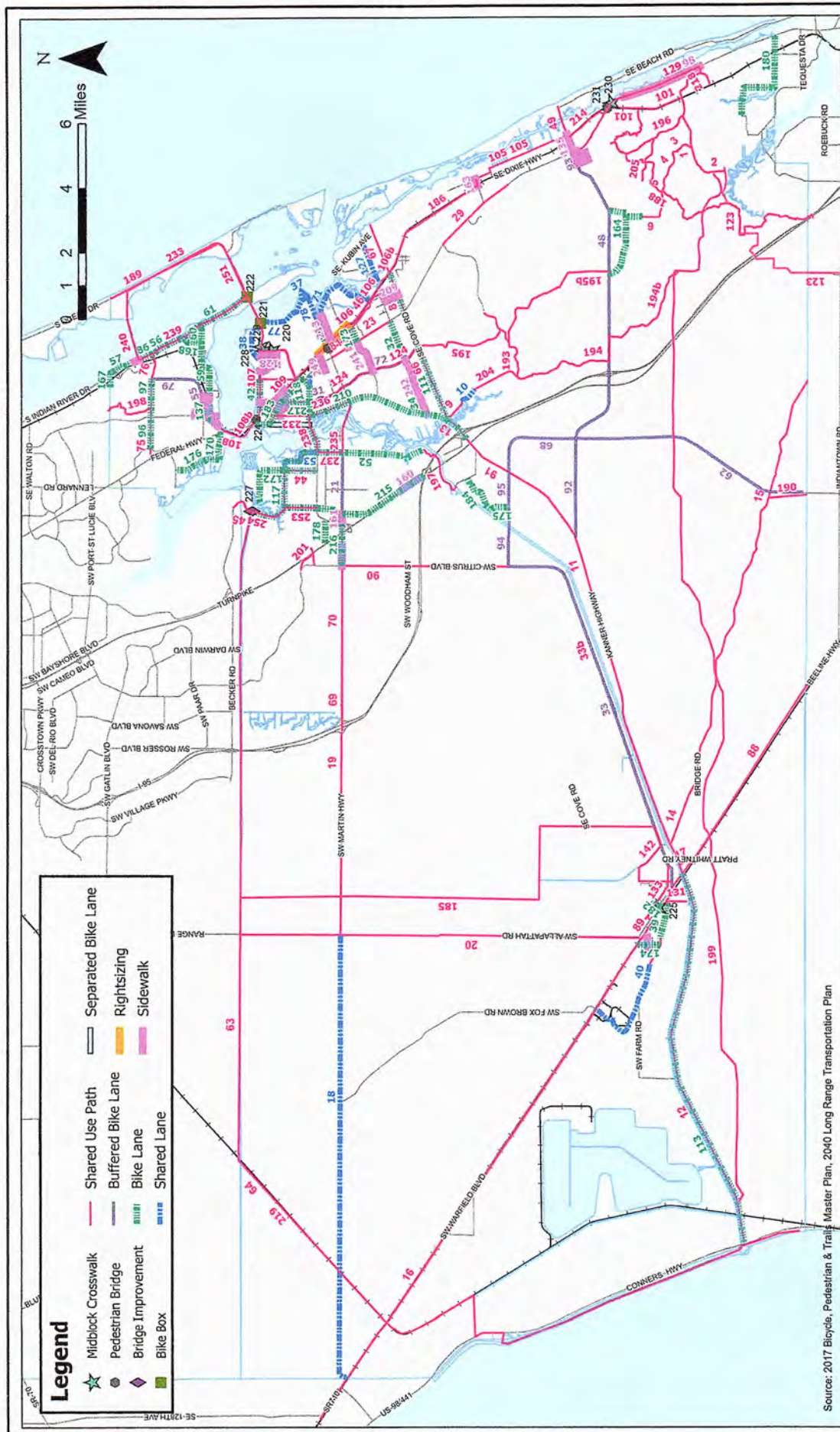
Retail	IN	OUT
Residential	295	251
	77	45

Years Grown: 6

APPENDIX A

Roadway Network Data/Non-Motorized and Transit Data





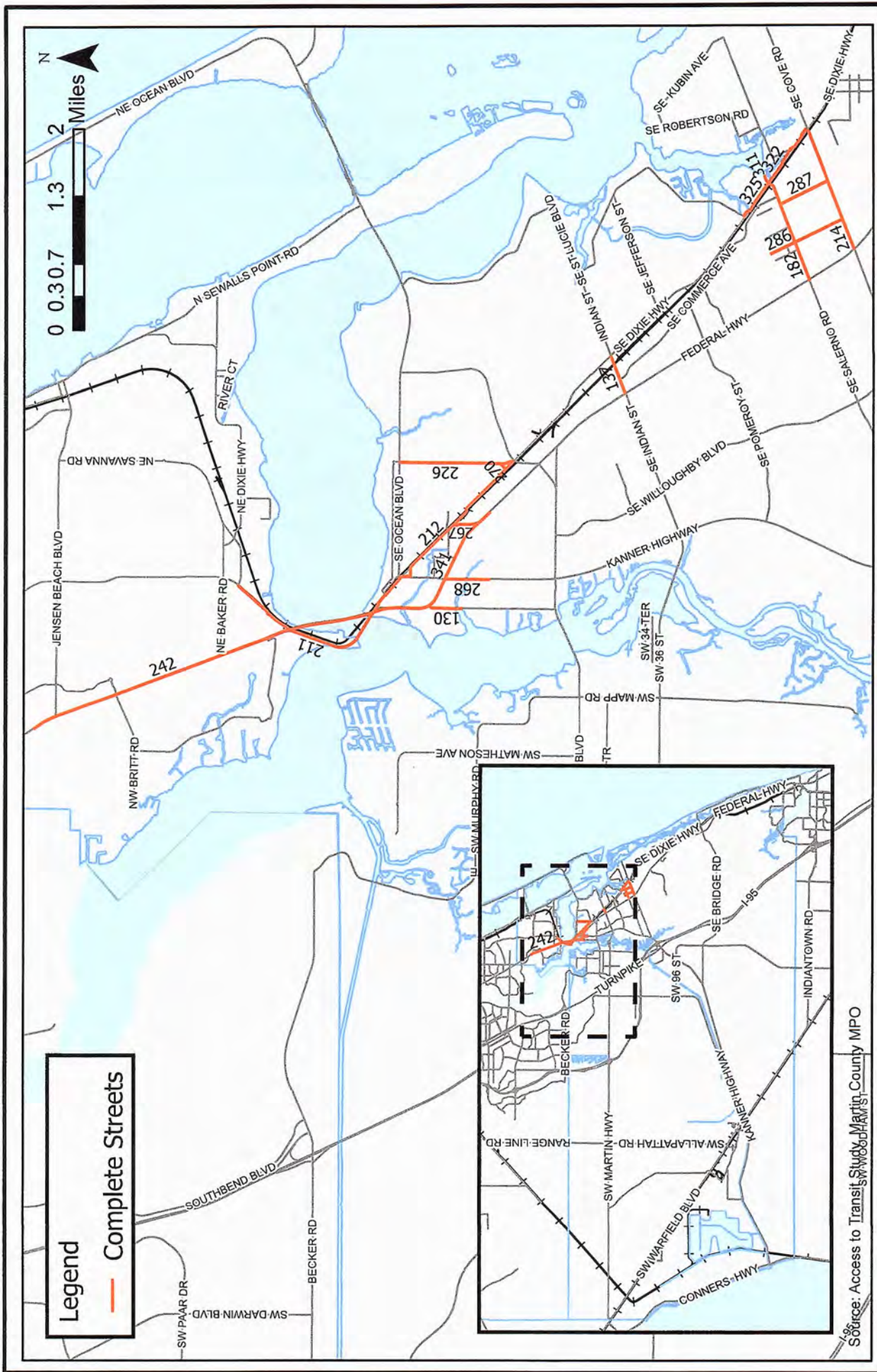


Figure 7-8

2045 Cost Feasible Plan Complete Streets Martin County



APPENDIX B

Pass-By Calculation Internal Capture Matrices

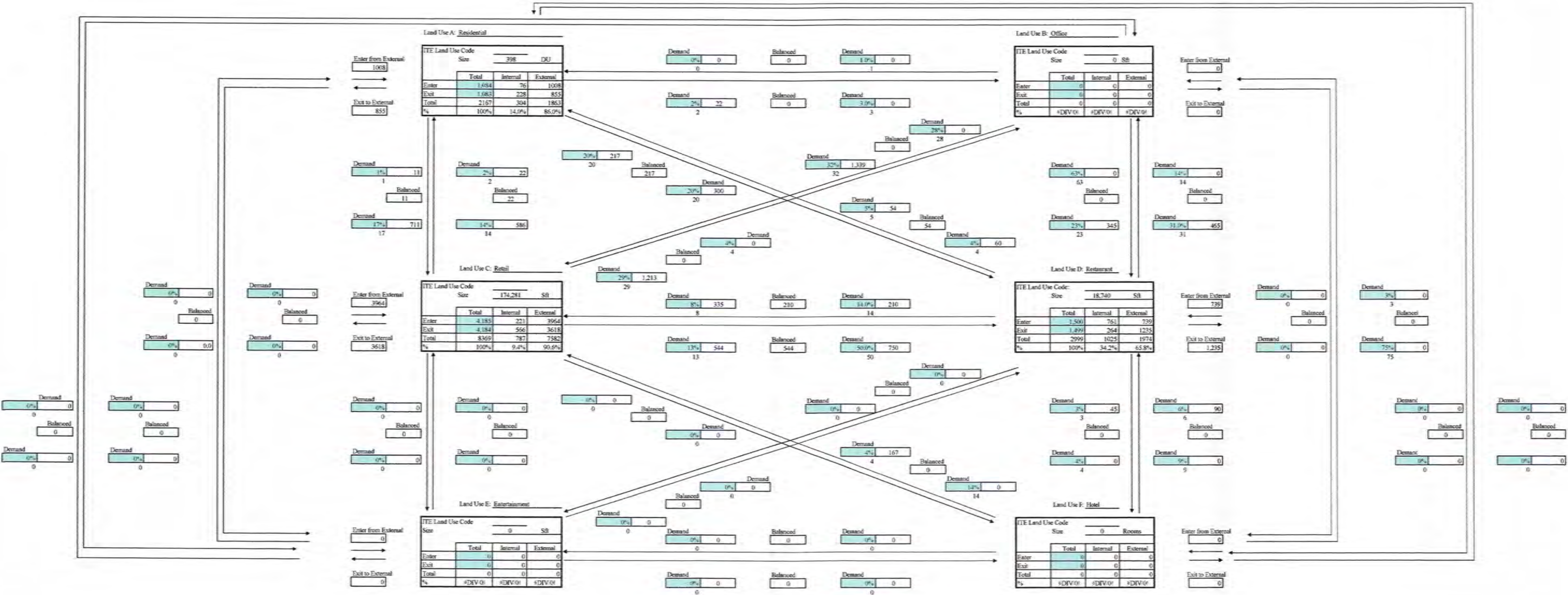
Allowable Pass-By Calculation

Segment	From	To	Existing 2019 AADT	Existing 2019 2-Way Peak Hour Trips	Growth Rate	2024 Daily Trips	Daily Pass-By Allowed	2024 2-Way Peak Hour Trips	Peak Hour Pass-By Allowed
Kanner Hwy	Indian St	Monterey Rd	21116	1631	1.005	21649	2164.9	1672	167.2
Willoughby Blvd	Indian St	Monterey Rd	11100	1280	1.037	13311	1331.1	1535	153.5
Indian St	Kanner Hwy	Willoughby Blvd	29306	2771	1.047	36871	3687.1	3486	348.6
Total:							7183		669

TABLE 1: Daily Internal Traffic

PROJECT
TRIP INTERNALIZATION - Daily

Name of Developer: PROJECT
Time Period: Daily



Net External Trips for Multi-Use Development									
	Land Use A	Land Use B	Land Use C	Land Use D	Land Use E	Land Use F			
Enter	1008	0	3964	739	0	0		5711	
Exit	855	0	3618	1235	0	0		5708	
Total	1863	0	7582	1974	0	0		11419	
Single-Use Trip Gen Estimate	2167	0	8369	2999	0	0		13535	15.6%

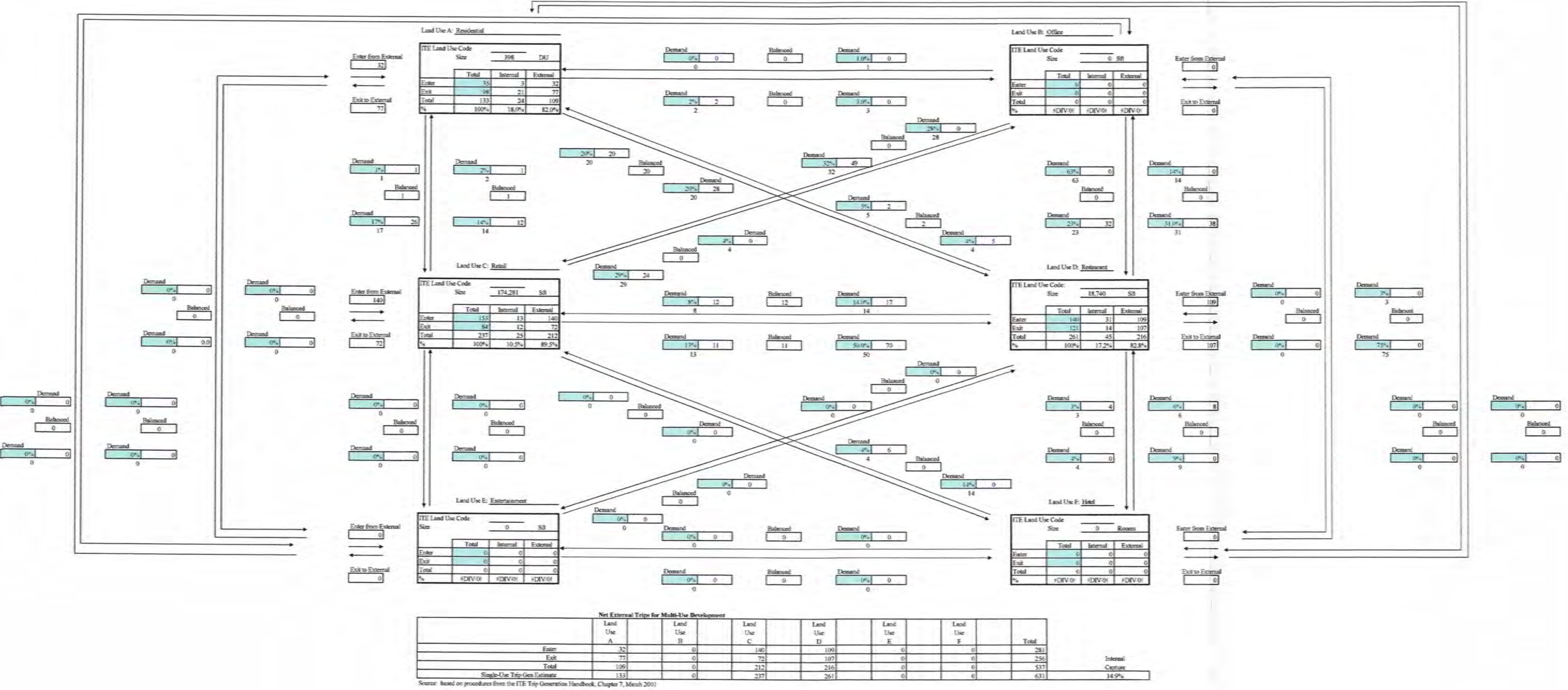
Source: based on procedures from the ITE Trip Generation Handbook, Chapter 7, March 2001

Note: Numbers outside of the Box are the ITE Pairing Values. Numbers inside the Box are Actual Values used.

TABLE 1: AM Internal Traffic

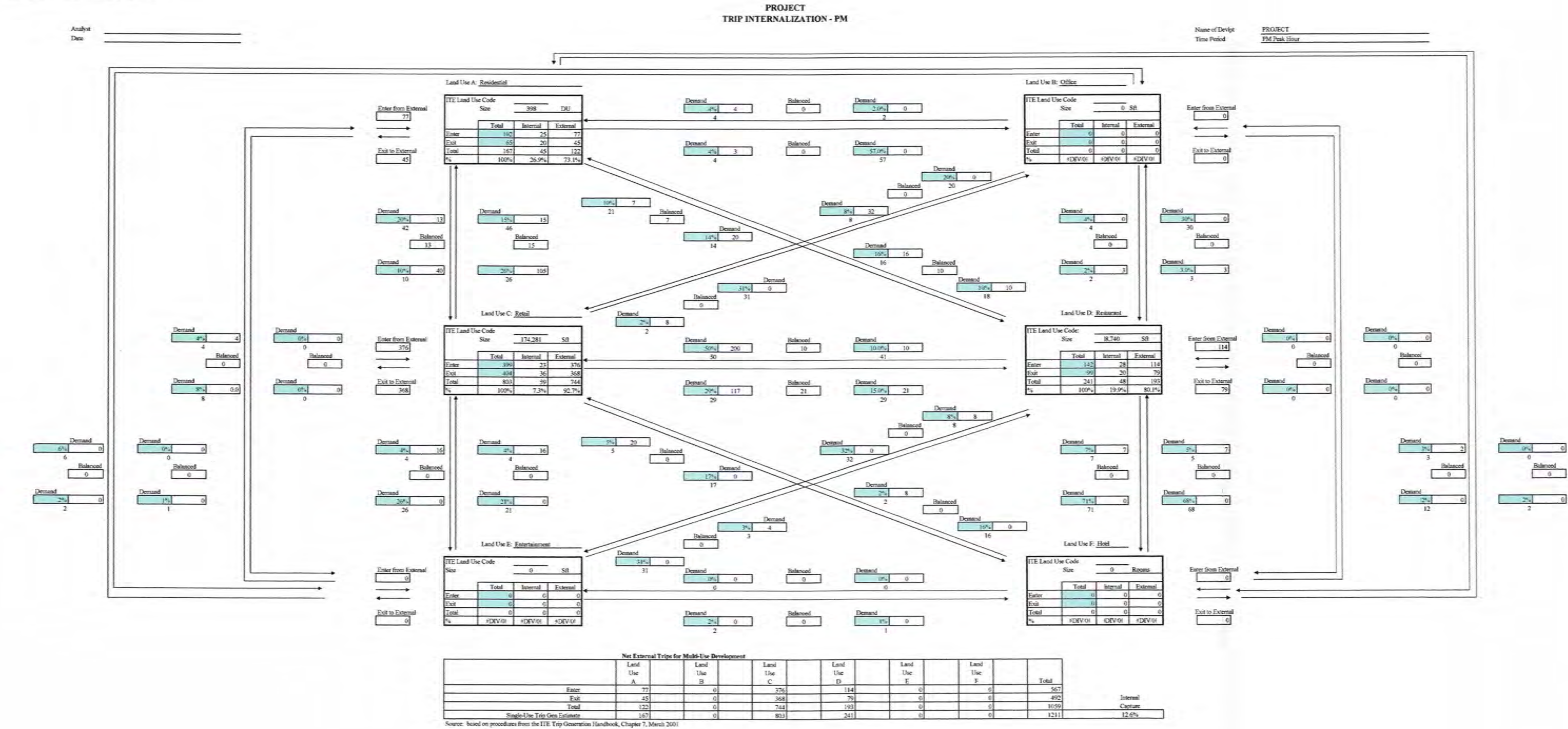
PROJECT
TRIP INTERNALIZATION - AM

Name of Developer
PROJECT
AM Peak Hour



Note: Numbers outside of the Box are the ITE Pairing Values. Numbers inside the Box are Actual Values used.

TABLE 1: PM Internal Traffic



APPENDIX C

Martin County 2040 Roadway Level of Service Inventory Report and Martin County 2019 Roadway Level of Service Inventory Report

Bold italic volume has been assumed or adjusted to account for recently constructed or committed projects.

Bold italic volume has been assumed or adjusted to account for recently constructed or committed projects. Shaded volume exceeds capacity of 1. CR volume and needs, additional analysis.

Bold italic volume has been assumed or adjusted to account for recently constructed or committed projects.

H11 = Bayesian Semant that does not assume in the Travel Demand Model (model H11)

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
CR-714 (Veteran's Memorial Bridge)	Mapp Rd	SR-76	Class I: 4-Ln Divided	2000	23,957	1,622	C	6.1%
CR-723 (Savanna Rd)	CR-707	NE 24th St	Class I: 2-Ln Undivided	880	9,135	384	C	0.5%
CR-723 (Savanna Rd)	NE 24th St	CR-707A (Jensen Beach Blvd.)	Class I: 2-Ln Undivided	880	9,631	471	C	1.0%
CR-726 (Citrus Blvd)	SR-710	Greenridge Ln	Uninterrupted Rural Hwy: 2-Ln Undivided	740	2,783	126	A/B	0.5%
CR-726 (Citrus Blvd)	Greenridge Ln	CR-76A	Uninterrupted Rural Hwy: 2-Ln Undivided	740	2,488	137	A/B	0.5%
CR-76A (Citrus Blvd.)	CR-726	SR-714	Transitional 2-Ln Uninter /Undivided Flow	1200	4,269	283	A/B	8.0%
CR-76A (SW 96th St)	CR-726	Pennsylvania Ave	Transitional 2-Ln Uninter /Undivided Flow	1200	3,845	195	A/B	8.0%
CR-76A (SW 96th St)	Pennsylvania Ave	SR-76	Class I: Transitional 2-Ln Undivided	800	8,086	492	C	2.5%
CR-A1A (Dixie Hwy)	SR-5	CR-708	Class II: 2-Ln Undivided	750	3,416	192	C	4.7%
CR-A1A (Dixie Hwy)	CR-708	Osprey St	Class I: 2-Ln Undivided	880	7,552	477	C	3.7%
CR-A1A (Dixie Hwy)	Osprey St	Heritage Blvd	2-Ln Uninter /Undivided Flow	1190	6,595	393	A/B	3.1%
CR-A1A (Dixie Hwy)	Heritage Blvd	Cove Rd	2-Ln Uninter /Undivided Flow	1190	7,147	346	A/B	2.9%
CR-A1A (Dixie Hwy)	Cove Rd	Salerno Rd	Class II: 2-Ln Divided	790	12,131	617	D	3.1%
CR-A1A (Dixie Hwy)	Salerno Rd	St. Lucie Blvd	Class II: 2-Ln Undivided	750	17,261	835	F	3.3%
CR-A1A (Dixie Hwy)	St Lucie Blvd	Jefferson St	Class II: 2-Ln Undivided	750	14,108	748	D	2.9%
CR-A1A (Dixie Hwy)	Jefferson St	Indian St	Class II: 4-Ln Divided	1630	18,389	848	D	2.2%
CR-A1A (Dixie Hwy)	Indian St	SR-714	Class I: 4-Ln Divided	2000	17,161	824	C	3.1%
CR-A1A (Dixie Hwy)	SR-714	SE Fifth St	2-Ln Undivided Non-State	675	6,710	346	D	1.6%
Farm Rd	Dr Martin Luther King Jr Dr	Palm Wy	Class II: 2-Ln Undivided	750	2,817	124	C	3.2%
Fox Brown Rd	SR-710	CR-714	Uninterrupted Rural Hwy: 2-Ln Undivided	740	347	16	A/B	0.0%
Goldenrod Rd	Britt Rd	SR-732	Class II: 4-Ln Divided	1630	4,810	278	A/B	0.5%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
Goldenrod Rd	SR-732	SR-5	Class II: 2-Ln Undivided	750	6,877	402	D	1.9%
Goldenrod Rd	SR-5	Westmoreland Blvd	Class II: 2-Ln Undivided	750	4,635	295	C	3.0%
Gomez Ave	CR-708	Crossrip St	Class II: 2-Ln Undivided	750	3,732	199	C	1.4%
Gomez Ave	Crossrip St	Osprey St	Class II: 2-Ln Undivided	750	1,065	61	C	0.5%
Green River Parkway	Dixie Hwy	Baker Rd	Class II: 2-Ln Undivided	750	6,833	351	C	3.1%
Green River Parkway	Baker Rd	SR-732	Class I: 2-Ln Undivided	880	8,341	596	C	4.3%
Green River Parkway	SR-732	St. Lucie County	2-Ln Uninter /Undivided Flow	1190	8,667	576	C	1.8%
Horseshoe Point Rd	CR-A1A	Kubin Ave	2-Ln Undivided Non-State	675	5,647	285	C	0.5%
Indian St	SR-76	Willoughby Blvd	Class I: 4-Ln Divided	2000	29,306	1,729	C	4.7%
Indian St	Willoughby Blvd	SR-5	Class I: 4-Ln Divided	2000	27,852	1,236	C	2.8%
Indian St	SR-5	Commerce Ave	Class I: 4-Ln Divided	2000	22,368	995	C	0.5%
Indian St	Commerce Ave	CR-A1A	Class I: 4-Ln Divided	2000	25,515	1,199	C	2.5%
Indian St	CR-A1A	St Lucie Blvd	2-Ln Undivided Non-State	675	7,051	346	D	1.1%
Indian River Dr	Palmer St	CR-707	Class II: 2-Ln Undivided	750	7,509	374	D	3.7%
Island Way	Palm Beach County	Jupiter Road	Transitional 2-Ln Uninter /Undivided Flow	1200	4,493	302	A/B	3.4%
Island Way	Jupiter Road	Country Club Dr	Class II: 2-Ln Undivided	750	4,855	213	C	0.5%
Jack James Rd	SR-76	Blue Water Wy	Class II: 2-Ln Undivided	750	3,132	294	C	0.5%
Lares St	CR-708	CR-A1A	2-Ln Undivided Non-State	675	3,349	235	C	0.6%
Little Club Wy	Country Club Dr	Wooden Bridge Wy	2-Ln Undivided Non-State	675	2,273	103	C	0.5%
Locks Rd	Canal St	SR-76	2-Ln Undivided Non-State	675	3,667	232	C	2.0%
MacArthur Blvd	Salfish Point	SR-A1A	2-Ln Undivided Non-State	675	5,989	368	D	6.8%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
SR-5 (US-1)	Britt Rd	SR-732	Class I: 7-Ln Divided	3530	55,878	2,898	C	0.8%
SR-5 (US-1)	SR-732	Westmoreland Blvd	Class I: 8-Ln Divided	4040	64,147	3,522	C	0.5%
SR-5 (US-1)	Westmoreland Blvd	St Lucie County	Class I: 8-Ln Divided	4040	60,343	2,721	C	0.5%
SR-710 (Warfield Blvd)	Okeechobee County	Fox Brown Rd	Uninterrupted Rural Hwy: 2-Ln Undivided	670	6,348	257	C	4.8%
SR-710 (Warfield Blvd)	Fox Brown Rd	CR-609 (Allapattah)	2-Ln Uninter /Undivided Flow	840	7,419	310	A/B	3.0%
SR-710 (Warfield Blvd)	CR-609 (Allapattah)	Van Buren	Class I: 2-Ln Undivided	870	12,335	694	C	4.0%
SR-710 (Warfield Blvd)	Van Buren	CR-726 (Citrus)	Class II: 4-Ln Divided	1710	12,335	694	A/B	4.0%
SR-710 (Warfield Blvd)	CR-726	SR-76 (Kanner)	Transitional 4-Ln Uninter /Divided Flow	2450	11,001	431	A/B	3.4%
SR-710 (Warfield Blvd)	SR-76	Palm Beach County	Transitional 4-Ln Uninter /Divided Flow	2450	6,959	258	A/B	1.4%
SR-714 (Martin Hwy)	I-95	CR-76A (Citrus)	Transitional 2-Ln Uninter /Undivided Flow	1200	13,786	735	C	5.1%
SR-714 (Martin Hwy)	CR-76A (Citrus)	Florida's Turnpike	Class I: 4-Ln Divided	2000	21,291	1,136	C	4.9%
SR-714 (Martin Downs Blvd)	Florida's Turnpike	CR-713	Class I: 4-Ln Divided	2000	20,695	1,038	C	1.7%
SR-714 (Martin Downs Blvd)	CR-713	Matheson Ave	Class I: 4-Ln Divided	2000	27,291	1,267	C	0.5%
SR-714 (Martin Downs Blvd)	Matheson Ave	Mapp Rd	Class I: 4-Ln Divided	2000	32,789	1,738	C	0.5%
SR-714 (Palm City Bridge)	Mapp Rd	SR-76	Class I: 4-Ln Divided	2000	34,751	1,989	D	0.5%
SR-714 (Monterey Rd)	SR-76	Willoughby Blvd	Class I: 4-Ln Divided	2000	22,465	1,274	C	0.5%
SR-714 (Monterey Rd)	Willoughby Blvd	Monterey Extension	Class I: 4-Ln Divided	2000	24,716	1,186	C	0.6%
SR-714 (Monterey Rd)	Monterey Extension	SR-5	Class I: 4-Ln Divided	2000	18,136	926	C	0.8%
SR-714 (Monterey Rd)	SR-5	CR-A1A	Class II: 4-Ln Divided	1630	21,333	1,099	D	0.5%
SR-714 (Monterey Rd)	CR-A1A	SR-A1A	Class I: 4-Ln Divided	1910	19,430	1,031	C	1.1%
SR-732 (Causeway Blvd)	CR-707	SR-A1A	2-Ln Uninter /Undivided Flow	1190	12,645	786	C	0.5%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SE) and Murphy Rd (PM/NB).

Martin County 2019 Roadway Level of Service Inventory Report

Road Name	From	To	Type	Generalized Service Capacity	2019 Average Annual Daily Traffic	2019 Peak Hour Directional Volume	2019 Generalized LOS	Avg. Annual Growth Rate
SR-732 (Jensen Beach Blvd)	SR-5	Green River Pkwy	Class I: 4-Ln Divided	2000	25,949	1,349	C	0.5%
SR-732 (Jensen Beach Blvd)	Green River Pkwy	CR-723	Class I: 4-Ln Divided	2000	23,804	1,154	C	0.5%
SR-76 (Kanner Hwy)	SR-15	SR-710	Uninterrupted Rural Hwy: 2-Ln Undivided	740	2,134	111	A/B	6.1%
SR-76 (Kanner Hwy)	SR-710	CR-708	Uninterrupted Rural Hwy: 2-Ln Undivided	740	3,412	151	A/B	0.9%
SR-76 (Kanner Hwy)	CR-708	CR-711/CR-76A	Transitional 2-Ln Uninter /Undivided Flow	1200	2,889	168	A/B	0.5%
SR-76 (Kanner Hwy)	CR-711/CR76A	Locks Rd	Class I: 4-Ln Divided	2000	13,401	831	C	1.7%
SR-76 (Kanner Hwy)	Locks Rd	Jack James	Class I: 4-Ln Divided	2000	19,231	920	C	0.5%
SR-76 (Kanner Hwy)	Jack James	Cove Rd	Class I: 6-Ln Divided	3020	43,880	2,372	C	0.6%
SR-76 (Kanner Hwy)	Cove Rd	Salerno Rd	Class I: 6-Ln Divided	3020	34,493	1,495	C	1.0%
SR-76 (Kanner Hwy)	Salerno Rd	Indian St	Class I: 6-Ln Divided	3020	30,912	1,293	C	2.6%
SR-76 (Kanner Hwy)	Indian St	SR-714	Class I: 6-Ln Divided	3020	21,116	993	C	0.5%
SR-76 (Kanner Hwy)	SR-714	SR-5	Class I: 6-Ln Divided	3020	25,158	1,006	C	0.5%
SR-A1A (Ocean Blvd)	SR-714	St Lucie Blvd	Class II: 4-Ln Divided	1630	19,437	1,081	D	0.6%
SR-A1A (Ocean Blvd)	St Lucie Blvd	Sewalls Point Rd	Class I: 4-Ln Divided	2000	23,060	1,127	C	1.0%
SR-A1A (Ocean Blvd)	Sewalls Point Rd	Macarthur Blvd	Class I: 2-Ln Divided	925	13,597	845	C	0.5%
SR-A1A (Ocean Blvd)	MacArthur Blvd	SR-732	2-Ln Uninter /Undivided Flow	1190	8,532	407	A/B	3.1%
SR-A1A (Ocean Blvd)	SR-732	St Lucie County	2-Ln Uninter /Undivided Flow	1190	14,752	870	D	1.8%
St Lucie Blvd	CR-A1A	Indian St	2-Ln Undivided Non-State	675	3,305	172	C	0.5%
St Lucie Blvd	Indian St	SR-A1A	2-Ln Undivided Non-State	675	7,524	429	D	3.3%
Westmoreland Blvd	St Lucie County	SR-5	Class II: 2-Ln Divided	790	12,271	601	D	1.0%
Willoughby Blvd	Cove Rd	Salerno Rd	Class I: 2-Ln Undivided	880	4,332	214	C	8.0%

Segments with shaded LOS require additional analysis.
The peaks are: CR-A1A (PM/SB) and Murphy Rd (PM/NB).